

CIVIL

KFC ENGINEERING

STRUCTURAL

SALAS O'BRIEN

MECHANICAL / ELECTRICAL



CJC

drawn by

BWB

checked by

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revisions

MOORE PUBLIC SCHOOLS
BOARD OF EDUCATION
MOORE, OKLAHOMA



DEMOLITION PACKAGE
CHILD CARE FACILITY
201 N. EASTERN AVE.

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- 5) THE FABRICATOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS. ANY SUCH ERECTION AIDS SHALL BE REMOVED FROM THE COMPLETED STRUCTURE IF DIRECTED BY THE OWNER'S REPRESENTATIVE.
- 6) ALL EXTENSION BARS, RUN-OFF PLATES, AND BACKING BARS USED IN WELDED CONNECTIONS SHALL BE REMOVED AND THE JOINTS SHALL BE GROUND SMOOTH WHERE SUCH CONNECTION IS PERMANENTLY EXPOSED TO VIEW OR IS DESIGNATED AS ARCHITECTURALLY EXPOSED STRUCTURAL STEEL.
- 7) HEADED STUDS AND DEFORMED BAR ANCHORS
- A) ALL HEADED STUDS AND DEFORMED BAR ANCHORS SHALL BE INSTALLED USING AUTOMATIC END-WELDING EQUIPMENT RECOMMENDED BY THE STUD OR ANCHOR MANUFACTURER. MANUAL WELDING OF HEADED STUDS OR DEFORMED BAR ANCHORS WILL NOT BE ALLOWED.
- B) IF A VISUAL INSPECTION REVEALS ANY STUD THAT DOES NOT SHOW A FULL 360-DEGREE FLASH OR ANY STUD THAT HAS BEEN REPAIRED BY MANUAL WELDING, SUCH STUD SHALL BE BENT TO AN ANGLE APPROXIMATELY 15 DEGREES FROM ITS ORIGINAL AXIS. THE DIRECTION OF BENDING FOR STUDS WITH LESS THAN A 360-DEGREE FLASH SHALL BE OPPOSITE TO THE MISSING PORTION OF THE FLASH.
- C) HEADED STUDS AND DEFORMED BAR ANCHORS THAT HAVE SUCCESSFULLY PASSED THE BEND TEST WITHOUT SIGN OF FAILURE SHALL BE ACCEPTABLE FOR USE AND LEFT IN THE BENT POSITION UNLESS DIRECTED OTHERWISE BY THE ENGINEER.
- D) WELDED STUDS NOT CONFORMING TO THE REQUIREMENTS OF AWS D1.1 SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR. THE CONTRACTOR SHALL REVISE THE WELDING PROCEDURE AS NECESSARY TO ENSURE THAT SUBSEQUENT STUD WELDING WILL MEET AWS D1.1 REQUIREMENTS.
- 8) STEEL EMBEDMENTS IN CONCRETE:
- A) ALL STEEL COMPONENTS TO BE EMBEDDED IN CONCRETE SHALL HAVE COATINGS AS DEFINED IN THE TABLE BELOW.

COATINGS FOR STEEL EMBEDMENTS IN CONCRETE		
EXPOSURE	FIELD WELDING	FINISH
EXTERIOR	EITHER	GALVANIZED
INTERIOR	YES	UNPAINTED
	NO	GALVANIZED

FOOTNOTES:

1. ALL WELDING TO PREVIOUSLY GALVANIZED COMPONENTS WILL REQUIRE REMOVAL OF THE GALVANIZING WITH GRINDING FOR AT LEAST 3-INCHES FROM EITHER SIDE OF THE INTENDED WELD AND ON BOTH SIDES OF THE WORKPIECE.
2. FIELD WELDED AREAS AND OTHER AREAS WITH REMOVAL OF, OR DAMAGE TO, THE GALVANIZED COATING SHALL HAVE THEIR COATING RESTORED IN ACCORDANCE TO ASTM A780, USING PAINT CONTAINING ZINC DUST OR SIMILAR PERMITTED PRODUCTS CAPABLE OF PROVIDING A MINIMUM ZINC-RICH COATING THICKNESS OF 2.0 MILS IN A SINGLE APPLICATION.

- B) IN ORDER TO REDUCE THE RISK OF HEAT-INDUCED CONCRETE SPALLING AT FIELD-WELDED EMBED PLATES:
- i) ALLOW SUPPORTING CONCRETE TO CURE FOR A MINIMUM OF 14-DAYS PRIOR TO FIELD WELDING.
- ii) PROVIDE THE WELD SIZE SHOWN IN DETAILS AND DO NOT OVER-WELD.
- 9) SHOP PRIMER
- A) ALL STEEL EXPOSED TO EXTERIOR WEATHER OR AN UNCONTROLLED ENVIRONMENT SHALL BE BLAST CLEANED AND PRIMED WITH A SUBMITTED AND APPROVED ZINC-RICH PRIMER.
- B) INTERIOR STEEL SHALL BE SHOP PRIMED WITH THE FABRICATORS STANDARD SHOP PRIMER.
- C) SHOP PRIMER SHALL NOT BE APPLIED TO THE FOLLOWING AREAS:
- i) SURFACES EMBEDDED IN CONCRETE OR MORTAR. EXTEND PRIMING OF PARTIALLY EMBEDDED MEMBERS TO A DEPTH OF 2 INCHES.
- ii) SURFACES TO BE FIELD WELDED.
- iii) SURFACES TO BE HIGH-STRENGTH BOLTED WITH SLIP-CRITICAL CONNECTIONS.
- iv) SURFACES TO RECEIVE SPRAYED FIRE-RESISTIVE MATERIALS.
- v) GALVANIZED SURFACES.

E. STEEL MISCELLANEOUS:

- 1) ALL EDGE ANGLES SUPPORTING ROOF OR FLOOR DECK SHALL BE CONTINUOUS BUTT-SPLICE WELDED OVER SUPPORTS.
- 2) ALL ELEVATED MECHANICAL EQUIPMENT SHALL BE SUPPORTED BY STEEL FRAMING. IF SPECIFIC FRAMING SIZES ARE NOT PROVIDED ON THE FRAMING PLAN, REFER TYPICAL DETAILS FOR ROOF AND FLOOR OPENING FRAME DETAILS.
- 3) SUBSTITUTION OF POST-INSTALLED ANCHORS FOR EMBEDDED ANCHORS SHOWN ON THE DRAWINGS WILL NOT BE PERMITTED UNLESS SPECIFICALLY APPROVED IN WRITING BY THE OWNER'S REPRESENTATIVE.
- 4) WHERE POST-INSTALLED ANCHORS ARE USED IN CONTINUOUS ANGLES, FABRICATE ANGLE WITH OPTIONAL HOLE LOCATIONS TO ALLOW REMEDIATION OF CASES WHERE ANCHORS FOUL WITH REBAR. AS AN EXAMPLE, FOR A CONTINUOUS ANGLE WITH ANCHORS AT 24" ON CENTER, PROVIDE HOLES AT 6" ON CENTER.
- 5) GALVANIZED LOOSE LEDGE ANGLES SHALL BE PROVIDED OVER ALL MASONRY VENEER OPENINGS OR RECESSES DEEPER THAN 1". LINTELS SHALL HAVE 1" OF BEARING AT EACH END FOR EVERY FOOT OF SPAN, WITH A MINIMUM OF 4" AND SIZED AS FOLLOWS UNLESS SHOWN OTHERWISE IN THE DRAWINGS.
- A) UP TO 4'-0" L3-1/2 x 3-1/2 x 3/8
- B) 4'-1" to 5'-0" L4 x 3-1/2 x 3/8 (LLV)
- C) 5'-1" to 6'-6" L5 x 3-1/2 x 3/8 (LLV)
- D) 6'-7" to 8'-0" L6 x 3-1/2 x 3/8 (LLV)

- H) $S_n = 0.504 \text{ IN}^3/\text{FT}$
- I) $F_y = 50 \text{ KSI}$
- J) SIDE LAPS: OVERLAPPED
- K) UNIT WEIGHT OF NORMAL WEIGHT CONCRETE FILL: 145 PCF

- 3) NON-COMPOSITE STRENGTH REQUIRED: THE COMPOSITE DECK SHALL BE CAPABLE OF SAFELY AND NON-COMPOSITELY SUPPORTING THE WET WEIGHT OF CONCRETE, INCLUDING AN ADDITIONAL CONCRETE PONDING WEIGHT OF 6 PSF DUE TO DEFLECTION OF DECK AND SUPPORTING STEEL FRAMING, PLUS TEMPORARY CONSTRUCTION LOADS WITHOUT REQUIRING SHORING. THE DECK SUPPLIER SHALL INFORM OWNER'S REPRESENTATIVE IF ANY PROJECT SPAN CONDITIONS DO NOT COMPLY WITH THIS REQUIREMENT AND SHALL RECOMMEND AREAS THAT MUST BE TEMPORARILY SHORED UNTIL CONCRETE HAS CURED FOR A MINIMUM OF 7 DAYS.
- 4) DECK ORIENTATION: COMPOSITE ROOF DECK SHALL BE PLACED WITH RIBS PERPENDICULAR TO SUPPORTING STEEL FRAMING EXCEPT AT SKEWED FRAMING MEMBERS.
- 5) SUPPORT FASTENERS: ENDS OF COMPOSITE METAL DECKING SHALL BE BUTTED AND CONNECTED TO SUPPORTS WITH MINIMUM 5/8" DIAMETER PUDDLE WELDS AT MAXIMUM 12" SPACING. (EACH SIDE OF BUTTED JOINT)
- 6) SIDE LAP FASTENERS: PROVIDE BUTTON PUNCHED SIDE LAPS 12" ON CENTER. SIDE LAP CONNECTIONS ARE DECK TO DECK CONNECTIONS.
- 7) MINIMUM BEARING LENGTH: MINIMUM EXTERIOR AND INTERIOR BEARING LENGTHS SHALL BE AS RECOMMENDED BY THE DECK MANUFACTURER FOR THE SPECIFIC DECK TYPE, GAGE AND SLAB THICKNESS USED.

- C. SUPPORTS FOR DECKING ARE DEFINED AS MEMBERS PROVIDING DIRECT TRANSVERSE SUPPORT AS WELL AS CONTINUOUS PARALLEL EDGE SUPPORT.

- D. ALL DECKING SHALL BE PLACED WITH RIBS PERPENDICULAR TO SUPPORTING ROOF OR FLOOR MEMBERS AND SHALL SPAN A MINIMUM OF 3 SPANS UNLESS SHOWN OTHERWISE IN STRUCTURAL DRAWINGS.

- E. METAL DECKING SHALL NOT BE USED TO SUPPORT ANY HANGING LOADS INCLUDING, BUT NOT LIMITED TO, SUSPENDED MECHANICAL, ELECTRICAL, OR PLUMBING EQUIPMENT, CABLE TRAYS OR RACEWAYS, CEILING FINISHES OR CEILING FRAMING.

- F. ALL DECK OPENINGS GREATER THAN 2-IN. SHALL BE SUPPORTED BY AN ANGLE FRAME. IF SPECIFIC FRAMING SIZES ARE NOT PROVIDED ON THE FRAMING PLAN, REFER TYPICAL DETAILS FOR ROOF OPENING FRAME DETAIL.

- G. PROVIDE SHEET STEEL COLUMN CLOSURES, Z-CLOSURES, CELL CLOSURES, POUR STOPS AND GIRDER FILLERS OF SAME MATERIAL AND FINISH AS DECK WITH THICKNESS AND PROFILE RECOMMENDED IN SDI FLOOR DECK DESIGN MANUAL, SECOND EDITION (JUNE 2020). WELD TO SUPPORTING STRUCTURE ACCORDING TO SDI RECOMMENDATIONS AND AS CONCEPTUALLY SHOWN IN TYPICAL FLOOR DECK CLOSURE DETAILS.

- H. ALL ROOF DECK EDGES SHALL BE SUPPORTED WITH POUR STOPS OR BENT PLATES. IF BENT PLATES ARE NOT SHOWN IN THE STRUCTURAL DRAWINGS, PROVIDE GAGE METAL POUR STOPS COMPLYING WITH SDI FLOOR DECK DESIGN MANUAL, SECOND EDITION (JUNE 2020), SECTION 5, TABLE 11 AND AS SHOWN IN TYPICAL DETAILS.

11) METAL DECK NOTES

- A. GOVERNING CODES AND STANDARDS: IN ADDITION TO THE REQUIREMENTS OF THE GOVERNING INTERNATIONAL BUILDING CODE, ALL STEEL DECK AND ASSOCIATED MATERIALS SHALL BE DETAILED, FABRICATED, AND ERRECTED IN ACCORDANCE WITH THE FOLLOWING STEEL DECK INSTITUTE (SDI) STANDARDS AND AS SUPPLEMENTED BY THESE GENERAL NOTES AND THE PROJECT DRAWINGS AND SPECIFICATIONS.
- 1) SDI NC-2017 "STANDARD FOR NONCOMPOSITE STEEL FLOOR DECK"
- 2) SDI RD-2017 "STANDARD FOR STEEL ROOF DECK"
- 3) SDI-C-2017 "STANDARD FOR COMPOSITE STEEL FLOOR DECK SLABS"
- 4) SDI-QA/QC-2017 "STANDARD FOR QUALITY CONTROL AND QUALITY ASSURANCE FOR INSTALLATION OF STEEL DECK"
- B. SHELTER COMPOSITE ROOF DECK:
- 1) COMPOSITE ROOF SYSTEM: COMPOSITE ROOF SYSTEM SHALL BE AS NOTED ON PLAN DRAWINGS AND AS SHOWN IN TYPICAL DETAILS.
- 2) BASIS OF DESIGN: VULCRAFT TYPE 3VLI COMPOSITE METAL DECK WITH THE CHARACTERISTICS AND STRUCTURAL PROPERTIES OUTLINED BELOW. COMPOSITE DECKS OF OTHER MANUFACTURERS ARE ACCEPTABLE IF THEY PROVIDE SIMILAR LOAD-CARRYING CAPACITY FOR THE DECK SPANS APPLICABLE TO THIS PROJECT.
- A) SDI DECK TYPE: COMPOSITE
- B) DEPTH: 2 IN.
- C) THICKNESS: 18 GAGE
- D) FINISH: GALVANIZED
- E) $I_p = 0.559 \text{ IN}^4/\text{FT}$
- F) $I_n = 0.558 \text{ IN}^4/\text{FT}$
- G) $S_p = 0.495 \text{ IN}^3/\text{FT}$

1. SHELTER GENERAL INFORMATION

- A. TYPE OF SHELTER: TORNADO.
- B. SHELTER WIND DESIGN GUIDELINES: ICC/NSSA STANDARD FOR THE DESIGN AND CONSTRUCTION OF STORM SHELTERS - 2014 (ICC 500-2014).
- C. ROOF SYSTEMS HAVE BEEN SELECTED IN ACCORDANCE WITH DEBRIS IMPACT TESTING IN ACCORDANCE WITH ASTM E 1886 AT TEXAS TECH UNIVERSITY (REFER SUMMARY REPORT DATED JUNE 2003 PREPARED BY WIND SCIENCE AND ENGINEERING RESEARCH CENTER). REFER SECTION A1 4" THICK CONCRETE-#4 REBAR REINFORCEMENT 12 INCHES ON CENTER EACH WAY (TESTED FOR 162 MPH)-67 MPH REQUIRED.
- D. WALL SYSTEMS HAVE BEEN SELECTED IN ACCORDANCE WITH DEBRIS IMPACT TESTING IN ACCORDANCE WITH ASTM E 1886 AT TEXAS TECH UNIVERSITY (REFER SUMMARY REPORT DATED JUNE 2003 PREPARED BY WIND SCIENCE AND ENGINEERING RESEARCH CENTER). REFER SECTION A1 8" REINFORCED CONCRETE WALLS (TESTED FOR 135 MPH) - 100 MPH REQUIRED.
- E. REFERENCE ELEVATION OF 100'-0" EQUALS DATUM FINISHED FLOOR ELEVATION OF 1246.09' FOR THE STORM SHELTER.
- F. BASED ON THE FLOOD INSURANCE RATE MAPS (FIRM) FOR CLEVELAND COUNTY (MAP NUMBER 4002C0160J) THE FINISHED FLOOR ELEVATIONS OF 1246.09' FOR THE STORM SHELTER IS GREATER THAN THE HIGHEST FLOOD ELEVATION OF 1239.0' WHICH HAS A 0.2-PERCENT ANNUAL CHANCE OF BEING EQUALED OR EXCEEDED IN ANY GIVEN YEAR. THE SHELTER IS NOT LOCATED IN AN AREA SUSCEPTIBLE TO FLOODING.
- G. REFER MECHANICAL DRAWINGS FOR LOCATIONS OF SHELTER VENTILATION.
- H. FURNISH LOOSE CONNECTION HARDWARE AND ANCHORAGE ITEMS TO BE EMBEDDED OR ATTACHED TO OTHER CONSTRUCTION BEFORE STARTING THAT WORK. PROVIDE LOCATIONS, SETTING DIAGRAMS, TEMPLATES, INSTRUCTIONS, AND DIRECTIONS, AS REQUIRED, FOR INSTALLATION.
- I. MISSILE CRITERIA (ICC-500 2014 TABLE 305.1.1)
 - 1) 15-LB SAWN LUMBER 2x4 MISSILE
 - 2) 100 MPH MISSILE SPEED FOR VERTICAL SURFACES
 - 3) 67 MPH MISSILE SPEED FOR HORIZONTAL SURFACES
- J. AFFIXED OPENING PROTECTIVE DEVICES HAVE BEEN DESIGNED USING 3/4" STEEL PLATE SELECTED IN ACCORDANCE WITH DEBRIS IMPACT TESTING IN ACCORDANCE WITH ASTM E 1886 AT TEXAS TECH UNIVERSITY (REFER REPORT NUMBER 20131123C - PROTOCOL 4 TESTING PROVIDED ON AN 11 GAGE STEEL PANEL PERFORMED ON 12/13/2013 AND 01/10/2014).
- K. SHELTER DOORS AND WINDOWS SHALL BE RATED FOR A MINIMUM DESIGN PRESSURE OF 246 PSF AND HAVE BEEN TESTED TO MEET THE IMPACT CRITERIA LISTED ABOVE. MANUFACTURER SHALL SUBMIT TESTING REPORTS AND INSTALLATION DETAILS TO ARCHITECT/ENGINEER FOR APPROVAL. THIS INCLUDES ANCHORAGE TYPES AND MINIMUM SETBACK DIMENSIONS FROM CONCRETE FACE.
- L. DOOR UNDERCUT SHALL NOT EXCEED 3/4" INCH.
- M. JOINTS, GAPS OR VOIDS IN THE SHELTER ENVELOPE THAT OPEN INTO THE PROTECTED OCCUPANT AREA INCLUDING MASONRY JOINTS, EXPANSION JOINTS, OPENING JOINTS SHALL BE A MAXIMUM OF 3/8" UNLESS NOTED OTHERWISE.

2. SHELTER DESIGN LOADS

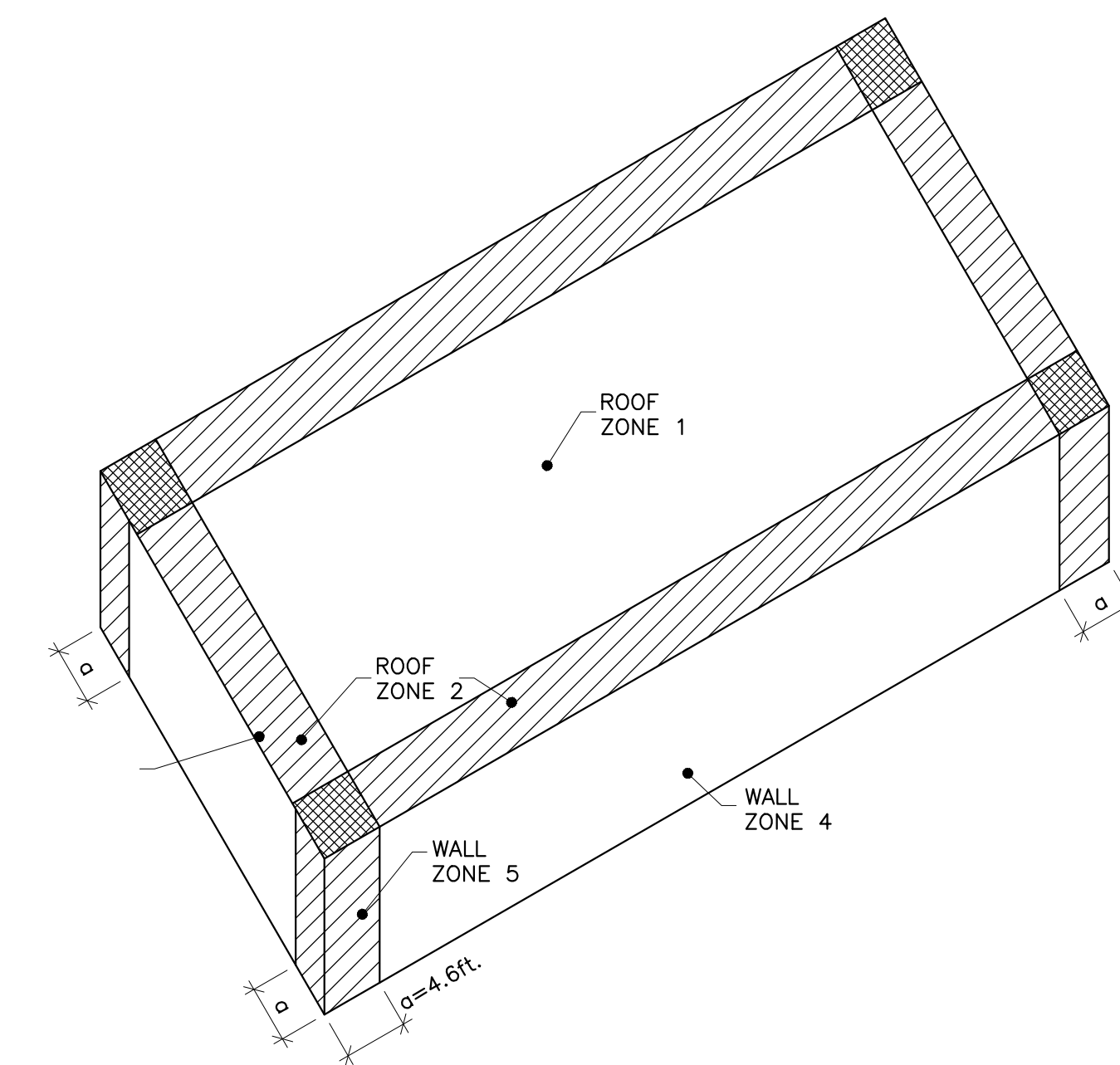
- A. DEAD LOAD: SELF WEIGHT OF MATERIALS, UNLESS NOTED OTHERWISE
- B. ROOF DEAD LOAD:
 - 1) CONCRETE DECK (4" NW CONC. ON 2" COMP. - 6" TOTAL).....63 PSF
 - 2) STEEL BEAMS.....7 PSF
 - 3) MISC (LIGHTING, DUCTWORK, PIPING, ETC.).....5 PSF
 - 4) TOTAL.....75 PSF
- C. LIVE LOADS:
 - 1) ROOF LIVE LOAD (SHELTER).....100 PSF
- D. SHELTER WIND PARAMETERS:
 - 1) GOVERNING CODE:.....ICC 500-2014
 - 2) IMPORTANCE FACTOR, Iw:.....1.0
 - 3) EXPOSURE CATEGORY:.....C
 - 4) INTERNAL PRESSURE COEFFICIENTS, PARTIALLY ENCLOSED, GCPI:.....+/-0.55
 - 5) TOPOGRAPHIC FACTOR, Kzt:.....1.0
 - 6) DIRECTIONALITY FACTOR, Kd:.....1.0
 - 7) WIND VELOCITY, V:.....250 MPH
- E. EXTREME WIND PRESSURES-MAIN WIND FORCE RESISTING SYSTEM:
 - 1) -GCFI
 - A) WINDWARD WALL:.....167 PSF IN
 - B) LEEWARD WALL:.....19 PSF OUT
 - C) SIDE WALL:.....6 PSF OUT
 - 2) +GCFI
 - A) WINDWARD WALL:.....18 PSF IN
 - B) LEEWARD WALL:.....132 PSF IN
 - C) SIDE WALL:.....156 PSF OUT
 - 3) ROOF 0'-0" TO 12'-6" FROM LEADING EDGE:.....179 PSF OUT
 - 4) ROOF 12'-6" TO 25'-0" FROM LEADING EDGE:.....132 PSF OUT
 - 5) ROOF BEYOND 25'-0" FROM LEADING EDGE:.....109 PSF OUT
 - 6) ROOF:.....54 PSF IN
- F. SNOW LOADS:
 - 1) GOVERNING CODE:.....ASCE 7-16
 - 2) IMPORTANCE FACTOR, Is:.....1.10
 - 3) GROUND SNOW LOAD, Pg:.....10 PSF
 - 4) EXPOSURE FACTOR, Ce:.....1.0
 - 5) THERMAL FACTOR, Ct:.....1.0
 - 6) ROOF SLOPE FACTOR, Cs:.....1.0
 - 7) CALCULATED FLAT ROOF SNOW LOAD, Pf:.....7.7 PSF
 - 8) MINIMUM FLAT ROOF SNOW LOAD, I*Pf:.....11 PSF
 - 9) RAIN ON SNOW SURCHARGE LOAD (3/8" PER FT > W/50):.....5 PSF
 - 10) DRIFT LOADS:.....ASCE 7-16
- G. SEISMIC DESIGN CRITERIA:
 - 1) GOVERNING CODE:.....ASCE 7-16
 - 2) IMPORTANCE FACTOR, Ie:.....1.25
 - 3) SOIL SITE CLASSIFICATION:.....C
 - 4) 0.2 SEC. MAPPED SPECTRAL ACCELERATION, Ss:.....0.328
 - 5) 1.0 SEC. MAPPED SPECTRAL ACCELERATION, S1:.....0.083
 - 6) SITE COEFFICIENT, 0.2 SEC. PERIOD, Fa:.....1.3
 - 7) SITE COEFFICIENT, 1.0 SEC. PERIOD, Fv:.....1.5
 - 8) 0.2 SEC. DESIGN SPECTRAL ACCELERATION, Sds:.....0.284
 - 9) 1.0 SEC. DESIGN SPECTRAL ACCELERATION, Sd1:.....0.083
 - 10) SEISMIC DESIGN CATEGORY:.....B
 - 11) SEISMIC PARAMETERS:
 - A) SEISMIC FORCE RESISTING SYSTEM: ORDINARY REINFORCED CONCRETE SHEAR WALLS
 - B) RESPONSE MODIFICATION COEFFICIENT, R:.....4.00
 - C) SYSTEM OVERSTRENGTH FACTOR, O:.....2.50
 - D) DEFLECTION AMPLIFICATION FACTOR, Cd:.....4.00
 - E) ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE METHOD.
 - F) SEISMIC RESPONSE COEFFICIENT, Cs:.....0.0888
 - G) TOTAL LATERAL BASE SHEAR, V:.....27 KIPS

3. QUALITY ASSURANCE PLAN

- A. DETAILED REQUIREMENTS: PER ICC 500, SECTION 107.3.1, DETAILS CONTAINED WITHIN THESE DOCUMENTS EXEMPLIFY THE FOLLOWING CRITERIA.
 - 1) ROOF CLADDING, SOFFITS AND ROOF FRAMING CONNECTIONS.
 - A) COMPOSITE ROOF DECK ON STEEL BEAMS DETAILED REQUIREMENTS:
 - 1. COMPOSITE DECK WELDED TO STEEL BEAMS
 - 2. HEADED STUDS CONNECTED TO STEEL BEAMS (THROUGH DECK) OR TO GIRDETS.
 - 3. MINIMUM 4" CONCRETE THICKNESS OVER COMPOSITE METAL DECK.
 - 4. REINFORCEMENT OF CONCRETE DECK WITH A MINIMUM OF #4 AT 12 INCHES ON CENTER EACH WAY.
 - 5. STEEL BEAM CONNECTIONS TO WALL WITH EMBED PLATE.
 - 6. STEEL BEAM CONNECTION TO BEAM SHEAR TAB PLATE.
 - 7. STEEL BEAM TO INTERIOR COLUMN CONNECTION.
 - 2) WALL CONNECTIONS TO ROOF DIAPHRAGM AND FRAMING.
 - A) DOWELS FROM CONCRETE WALL INTO SLAB.
 - B) EMBED PLATES IN CAST-IN-PLACE WALLS TO DECK ANGLES.
 - 3) ROOF DIAPHRAGM SYSTEM, INCLUDING CONNECTORS, DRAG STRUTS AND BOUNDARY ELEMENTS.
 - A) CAST-IN-PLACE SLAB WITH MINIMUM #4 AT 12 INCHES ON CENTER AND MINIMUM CHORD STEEL OF 1 #5 BARS.
 - 4) MAIN WIND FORCE RESISTING SYSTEMS INCLUDING BRACED FRAMES, MOMENT FRAMES AND SHEAR WALLS.
 - A) ORDINARY REINFORCED CAST-IN-PLACE CONCRETE SHEAR WALLS
 - 5) MAIN WIND FORCE RESISTING SYSTEM CONNECTIONS TO THE FOUNDATION.
 - A) REINFORCING STEEL DOWELS.
 - B) ANCHOR BOLTS.
 - C) GROUT.
 - 6) FABRICATION AND INSTALLATION OF COMPONENTS AND ASSEMBLIES OF THE SHELTER ENVELOPE REQUIRED TO MEET MISSILE IMPACT TEST REQUIREMENTS OF CHAPTER 3.
 - A) REFER TO ITEMS 1.D, 1.E, AND 1.F INDICATED ABOVE.
 - B) VERIFY DOOR TESTING AND ANCHORAGE REQUIREMENTS.
 - C) VERIFY SHUTTER TESTING AND ANCHORAGE REQUIREMENTS.
 - D) VERIFY OVERHEAD DOOR TESTING AND ANCHORAGE REQUIREMENTS.
 - E) VERIFY LOUVER TESTING AND ANCHORAGE REQUIREMENTS.
 - 7) WALL CLADDING AND WALL CLADDING CONNECTIONS.
 - A) CAST-IN-PLACE - VERTICAL AND HORIZONTAL REINFORCING.
 - 8) CORROSION RESISTANCE OR PROTECTION OF EXPOSED METAL CONNECTORS PROVIDING LOAD PATH CONTINUITY.
 - A) GALVANIZED EMBED PLATES.
 - B) ALL WELDS TO GALVANIZED EMBED PLATES MUST BE PROPERLY PREPPED PRIOR TO WELDING, SLAG REMOVED, WELD CLEANED AND COLD GALVANIZING PAINT ADDED.
 - 9) FOUNDATION DESIGN:
 - A) SHALLOW CONT. AND SPREAD FOUNDATION SYSTEM WITH SLIDING AND PASSIVE RESISTANCE.
 - B) SLAB-ON-GRADE NOT USED AS DIAPHRAGM AND SUBGRADE DRAG.
- B. MAIN WINDFORCE-RESISTING SYSTEMS AND WIND-RESISTING COMPONENTS
 - 1) SHALLOW CONT./SPREAD FOOTING
 - 2) CONCRETE BEARING/SHEAR WALLS
 - 3) COMPOSITE STEEL BEAMS
 - 4) STEEL EMBEDS AND PERIMETER ANGLES
 - 5) COMPOSITE METAL DECK
 - 6) REINFORCING STEEL
 - 7) STEEL PROTECTION DEVICES
 - 8) SHELTER ROOM DOORS, WINDOWS, AND LOUVERS.
- C. REQUIRED SPECIAL INSPECTIONS SHALL BE PROVIDED FOR CONSTRUCTION AND INSTALLATION OF MATERIALS AS REQUIRED BY THE AUTHORITY HAVING JURISDICTION IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE. TYPE AND FREQUENCY ARE IDENTIFIED ON S1.05.
 - 1) ANCHORAGE OF ALL OPENING PROTECTIVE DEVICES (DOORS, WINDOWS, SHUTTERS, LOUVERS, AND AFFIXED COWLINGS) SHALL BE CONTINUOUSLY OBSERVED BY A SPECIAL INSPECTOR.
- D. INSPECTION OF FABRICATORS: WHERE FABRICATION OF STRUCTURAL LOAD-BEARING AND DEBRIS-IMPACT-RESISTANT COMPONENTS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATOR SHOP, SPECIAL INSPECTION OF THE FABRICATOR SHALL BE PROVIDED.
 - 1) EXCEPTION: PREFABRICATED STORM SHELTER COMPONENTS THAT HAVE BEEN INSPECTED AND LABELED BY AN APPROVED AGENCY MEETING THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE.
- E. STRUCTURAL OBSERVATIONS: IN ADDITION TO THE REQUIREMENTS OF THE SPECIAL INSPECTOR, THE OWNER SHALL EMPLOY A LICENSED PROFESSIONAL STRUCTURAL ENGINEER TO INSPECT THE FOLLOWING ELEMENTS OF THE MAIN WIND FORCE RESISTING SYSTEM OF THE SHELTER TO VERIFY CONFORMANCE WITH THE CONTRACT DOCUMENTS AT SIGNIFICANT CONSTRUCTION STAGES AND AT THE COMPLETION OF THE STRUCTURAL SYSTEM. DEFICIENCIES SHALL BE REPORTED IN WRITING TO THE OWNER AND AHJ. AT THE CONCLUSION OF THE WORK, THE REGISTER DESIGN PROFESSIONAL SHALL SUBMIT TO THE AHJ A WRITTEN STATEMENT INDICATING SITE VISITS MADE AND IDENTIFYING ANY OUTSTANDING DEFICIENCIES THAT HAVE NOT BEEN RESOLVED.
 - 1) FOUNDATIONS
 - A) REBAR SIZE, SPACING AND LAP LENGTHS AND LOCATIONS
 - 2) WALLS
 - A) REBAR SIZE, SPACING, LAP LENGTHS
 - B) EMBED SIZE AND LOCATIONS
 - 3) ROOF
 - A) REBAR SIZE, SPACING, LAP LENGTHS
 - B) PERIMETER AND OPENING DECK EDGE INSTALLATION
 - C) METAL DECK SIZE AND ATTACHMENT
 - D) HEADED STUD ANCHORS SIZE, SPACING, AND ATTACHMENT
 - 4) DOORS
 - A) VERIFY INSTALLATION OF EACH DOOR IS PER MANUFACTURER'S TESTED ASSEMBLY.
 - B) VERIFY ALL DOOR HARDWARE IS FULLY OPERATIONAL.
 - C) VERIFY DOORS HAVE PROPER LABELS INDICATING REQUIRED PRESSURE AND IMPACT RATING.
 - 5) PROTECTION DEVICES
 - A) VERIFY INSTALLATION OF EACH STEEL PLATE PROTECTION DEVICES IS PER CONTRACT DRAWINGS.
 - B) VERIFY ALL OPENINGS REQUIRING PROTECTION HAVE PROTECTION DEVICE INSTALLED.
- F. CONTRACTOR RESPONSIBILITY: EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION, FABRICATION OR INSTALLATION OF A MAIN WIND FORCE-RESISTING SYSTEM OR ANY COMPONENT LISTED IN THE QUALITY ASSURANCE PLAN SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE AHJ, THE ARCHITECT/ENGINEER, AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON THE SYSTEM OR COMPONENT. CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL BE INCLUDED WITH THE FIRST SUBMITTAL/SHOP DRAWING. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN:
 - 1) ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE QUALITY ASSURANCE PLAN.
 - 2) ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.
 - 3) PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF REPORTS.
 - 4) IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

G. PRE-CONSTRUCTION MEETING: TO BE HELD PRIOR TO STARTING CONSTRUCTION AN

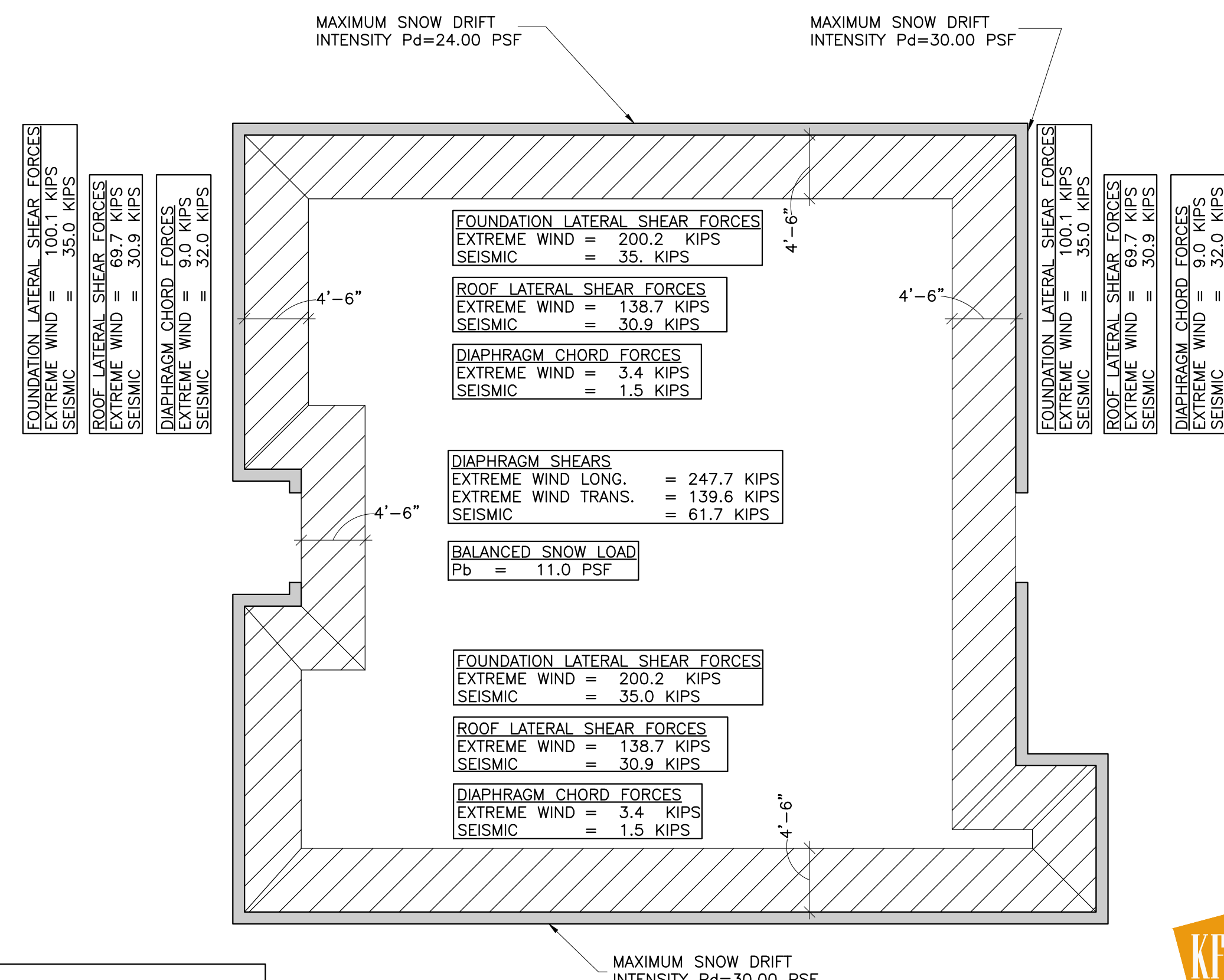
- INCLUDE THE CONSTRUCTION MANAGER, DESIGN TEAM, SPECIAL INSPECTOR, STRUCTURAL OBSERVER, AND THE CONTRACTORS RESPONSIBLE FOR FABRICATION AND INSTALLATION OF THE MAIN WIND FORCE-RESISTING SYSTEM AND COMPONENTS. AT A MINIMUM THE MEETING SHALL CONSIST OF THE FOLLOWING ITEMS:
 - 1) REVIEW THE CONSTRUCTION DOCUMENTS AND QUALITY ASSURANCE PLAN.
 - 2) REVIEW THE CONTRACTORS STATEMENT OF RESPONSIBILITY.
 - 3) REVIEW THE CONSTRUCTION SCHEDULE AND SEQUENCE OF REQUIRED SPECIAL INSPECTIONS.
- H. SHELTER PENETRATIONS: THE DESIGN TEAM, CONSTRUCTION MANAGER, MEP TRADES, PRECAST PROVIDER, AND SPECIAL INSPECTOR SHALL MEET TO IDENTIFY ALL ITEMS PENETRATING THE PRECAST SHELTER ENVELOPE. THE METHOD OF INSTALLATION (FIELD CORING VS. FORMING) AND APPROPRIATE PROTECTION DETAILS SHALL BE REVIEWED FOR EACH INSTANCE.
- I. SHELTER SPECIAL INSPECTION
 - 1) POST INSTALLED ANCHORAGE AT THE SHELTER IN CONCRETE SHALL BE CONTINUOUSLY INSPECTED.
 - 2) INSTALLATION OF SHELTER DOORS OR AFFIXED COWLINGS SHALL BE CONTINUOUSLY INSPECTED.
 - 3) WELDING OF SHELTER DOORS OR AFFIXED COWLINGS SHALL BE CONTINUOUSLY INSPECTED.
 - 4) HEADED STUD ANCHORS AND THE QUALITY OF THEIR CONNECTION TO THE TOP OF BEAM FLANGES
 - 5) COMPOSITE METAL DECK SIZE AND ATTACHMENT



1 COMPONENT AND CLADDING ISOMETRIC
S103 SCALE: 1/16"=1'-0"

NOTE: POSITIVE PRESSURES ARE DIRECTED INWARD ON THE EXTERIOR SURFACE. NEGATIVE PRESSURES ARE DIRECTED OUTWARD ON THE EXTERIOR SURFACE.

EXTREME WIND COMPONENTS AND CLADDING LOADS										
BUILDING ELEMENT	SPAN (FT)	WIDTH (FT)	AREA (FT ²)	WIND PRESSURE ZONE 1 (PSF)	WIND PRESSURE ZONE 2 (PSF)	WIND PRESSURE ZONE 3 (PSF)	WIND PRESSURE ZONE 4+2 (PSF)	WIND PRESSURE ZONE 5+3 (PSF)	WIND PRESSURE ZONE 4 (PSF)	WIND PRESSURE ZONE 5 (PSF)
14'-0 3/4" WALL	14.02	4.71	66						-192/180	-212/180
12'-8" WALL	12.67	4.26	54						-194/181	-215/181
WINDWARD + LEEWARD PARAPET	3.50	1.17	4				461	585		
ROOF BEAM	43.30	14.40	623	-197/102	-224/102	-224/102				
ROOF DECK	6.67	2.22	15	-209/114	-307/114	-425/114				
STORM DOOR	-	-	10						-209/197	-246/197



NOTES:
1. VERIFY FINAL LOCATION OF ALL ROOF PENETRATIONS AND HUNG EQUIPMENT.
2. REFER ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ADDITIONAL HANGING LOADS NOT SHOWN.

2 SHELTER LOAD PLAN
S103 SCALE: 1/8"=1'-0"

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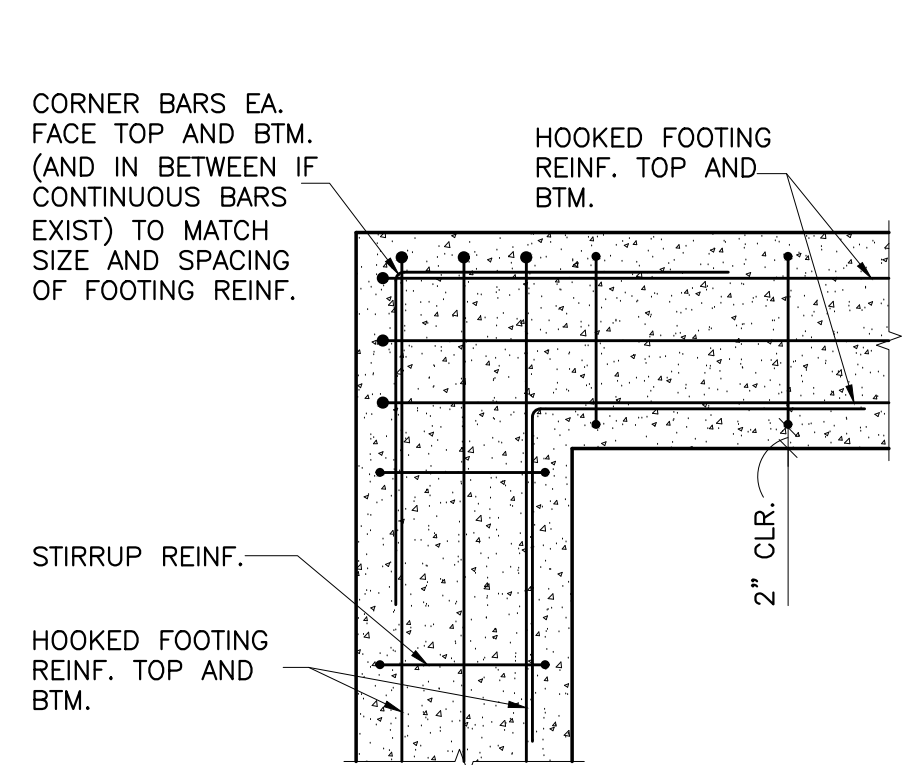
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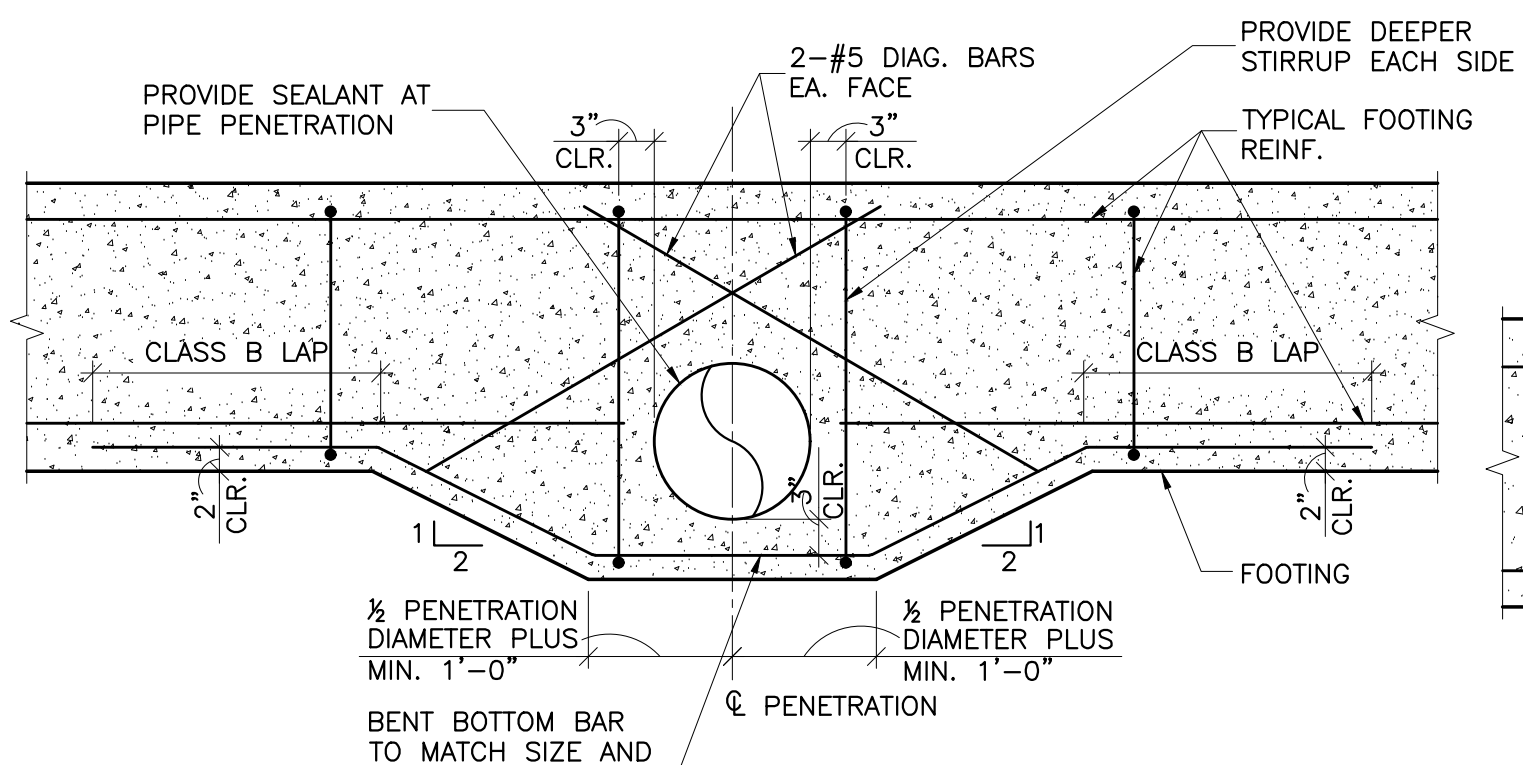
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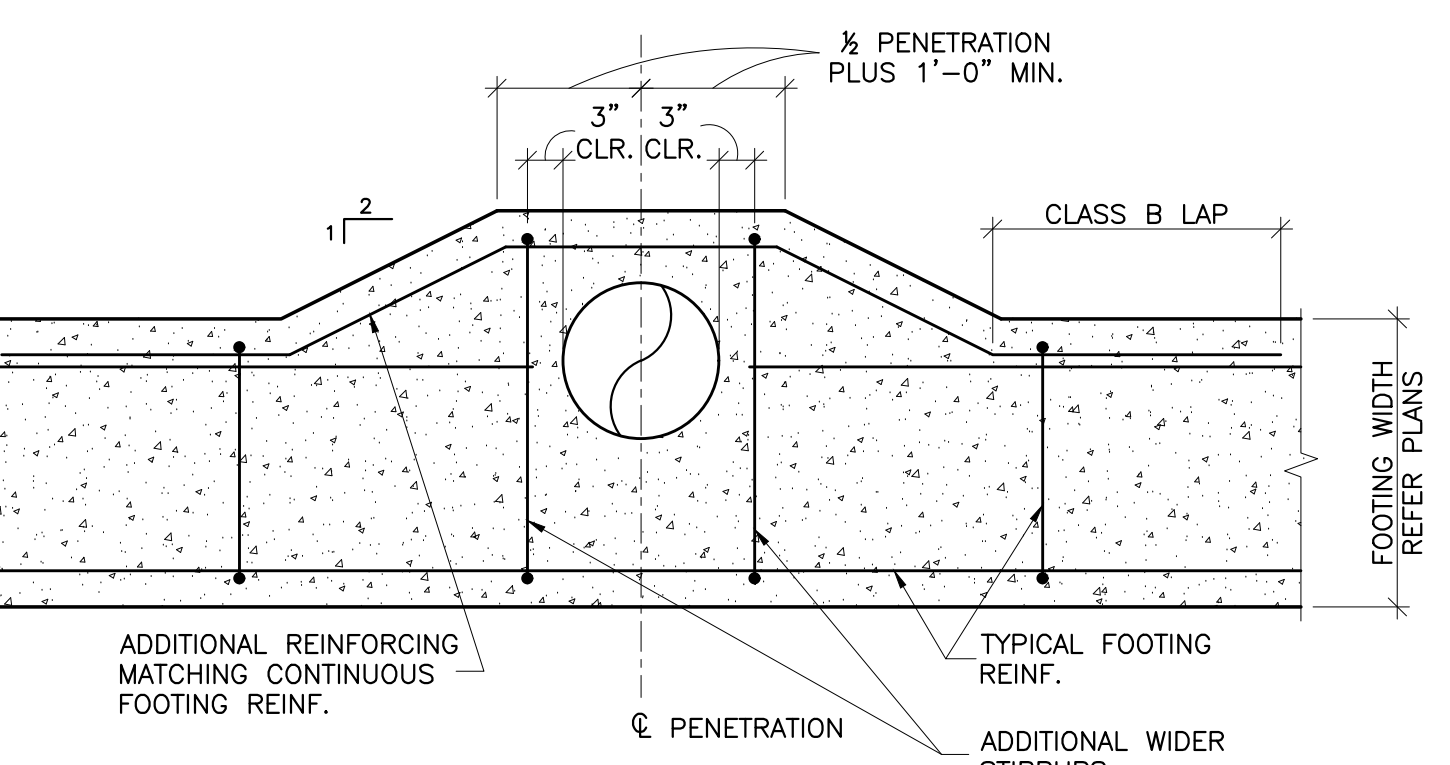
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Structural Engineering
OK CA #3888, EXP. 06/30/25
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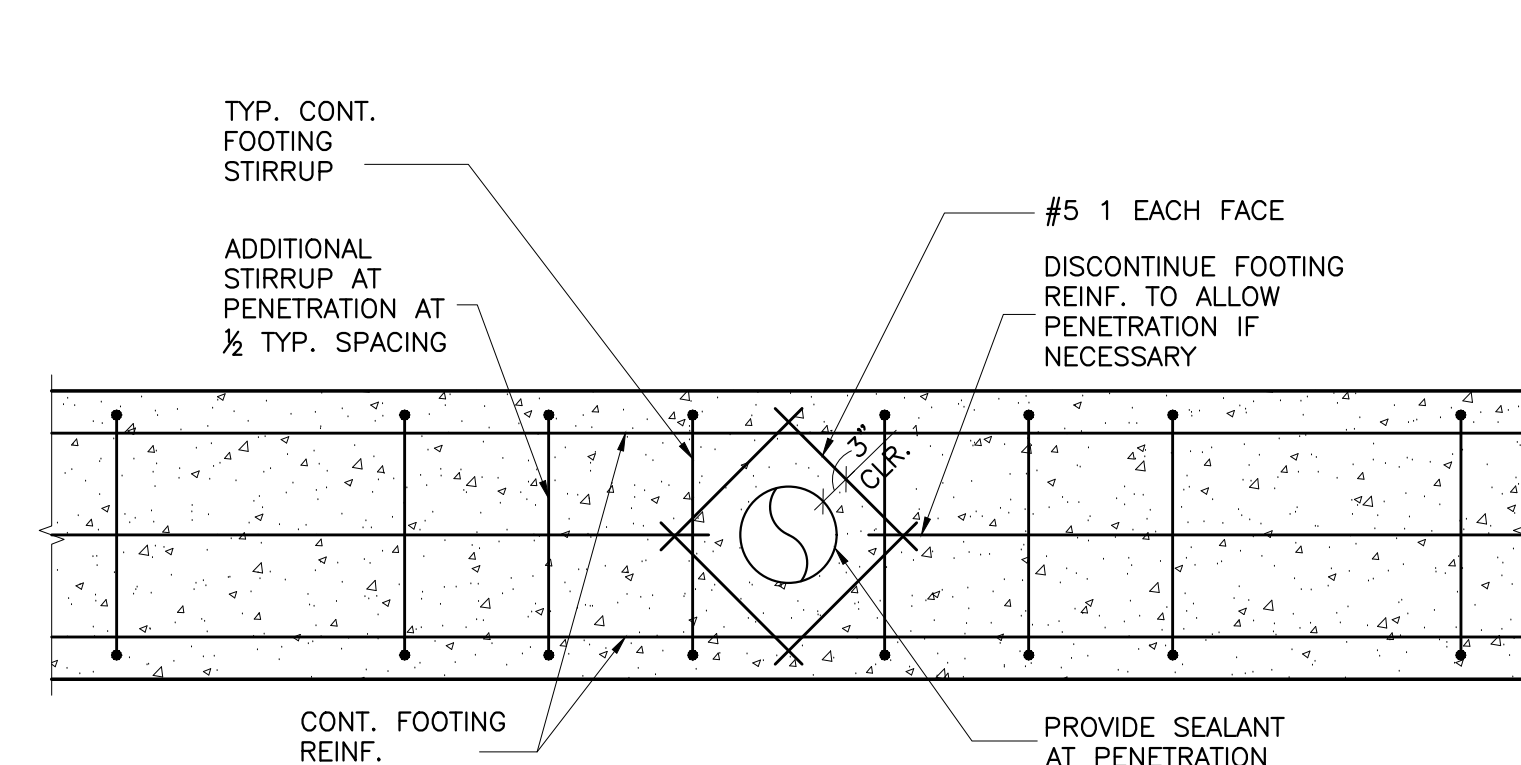
1 TYP. FOOTING CORNER REINF.
SCALE: NONE



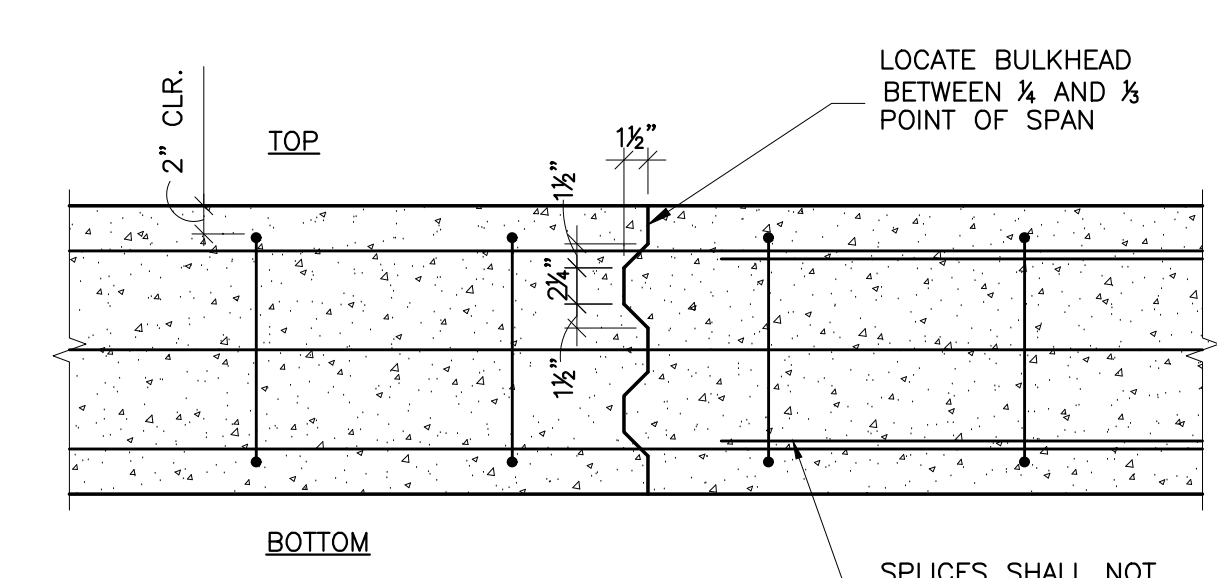
2 TYP. FOOTING PENETRATION
SCALE: NONE



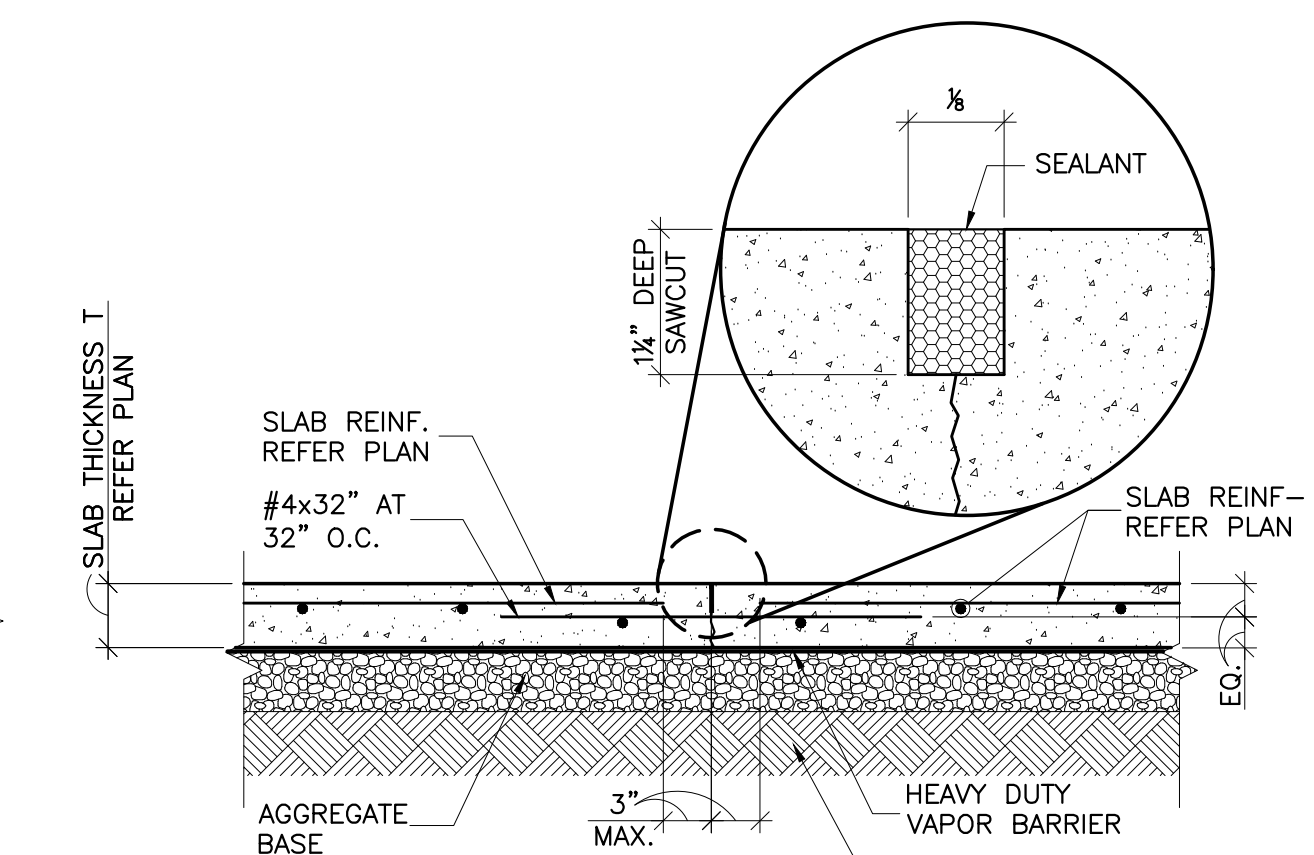
3 PLAN SECTION AT TYPICAL VERTICAL PENETRATION
SCALE: NONE



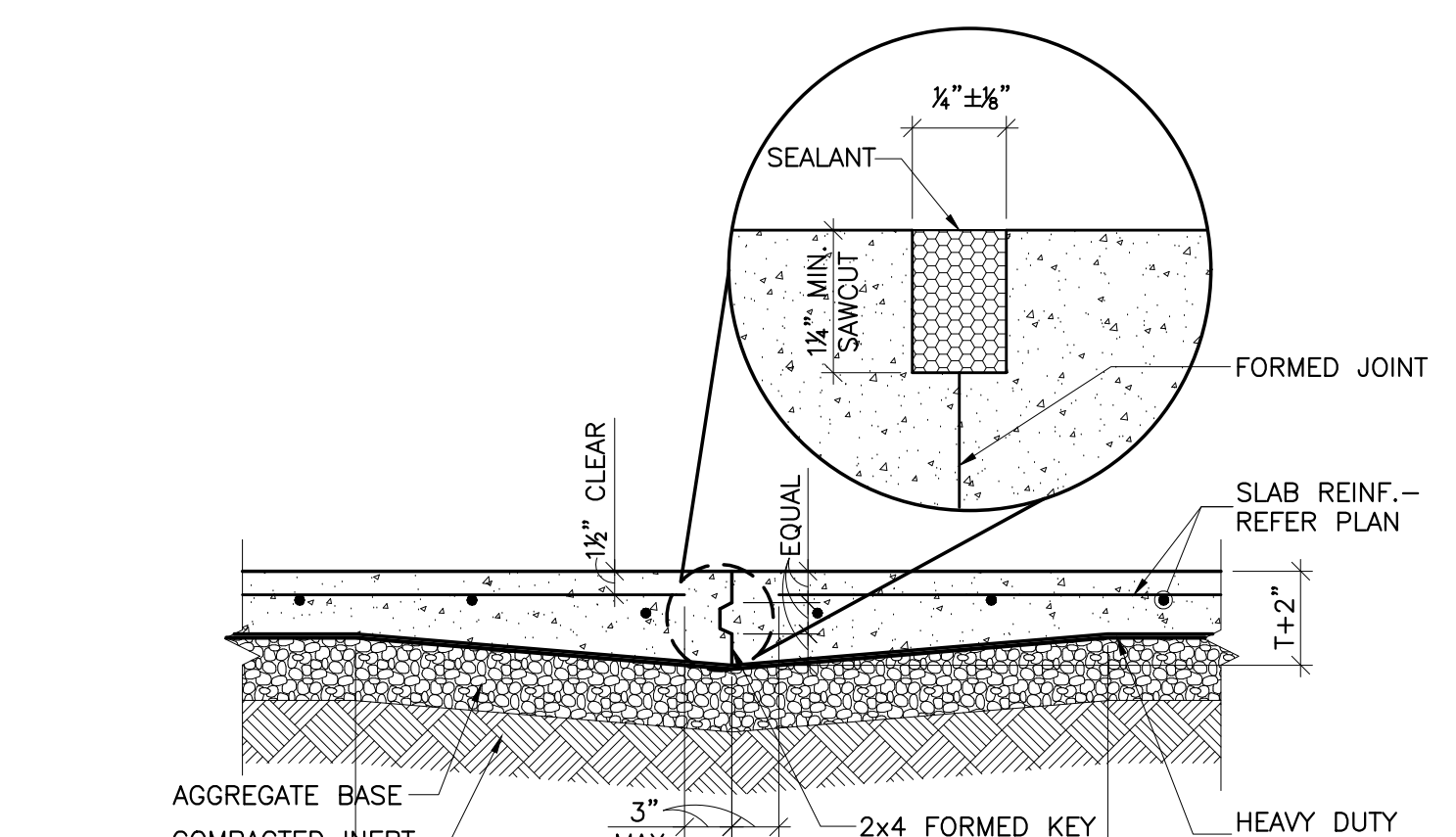
4 FOOTING PENETRATION
SCALE: NONE



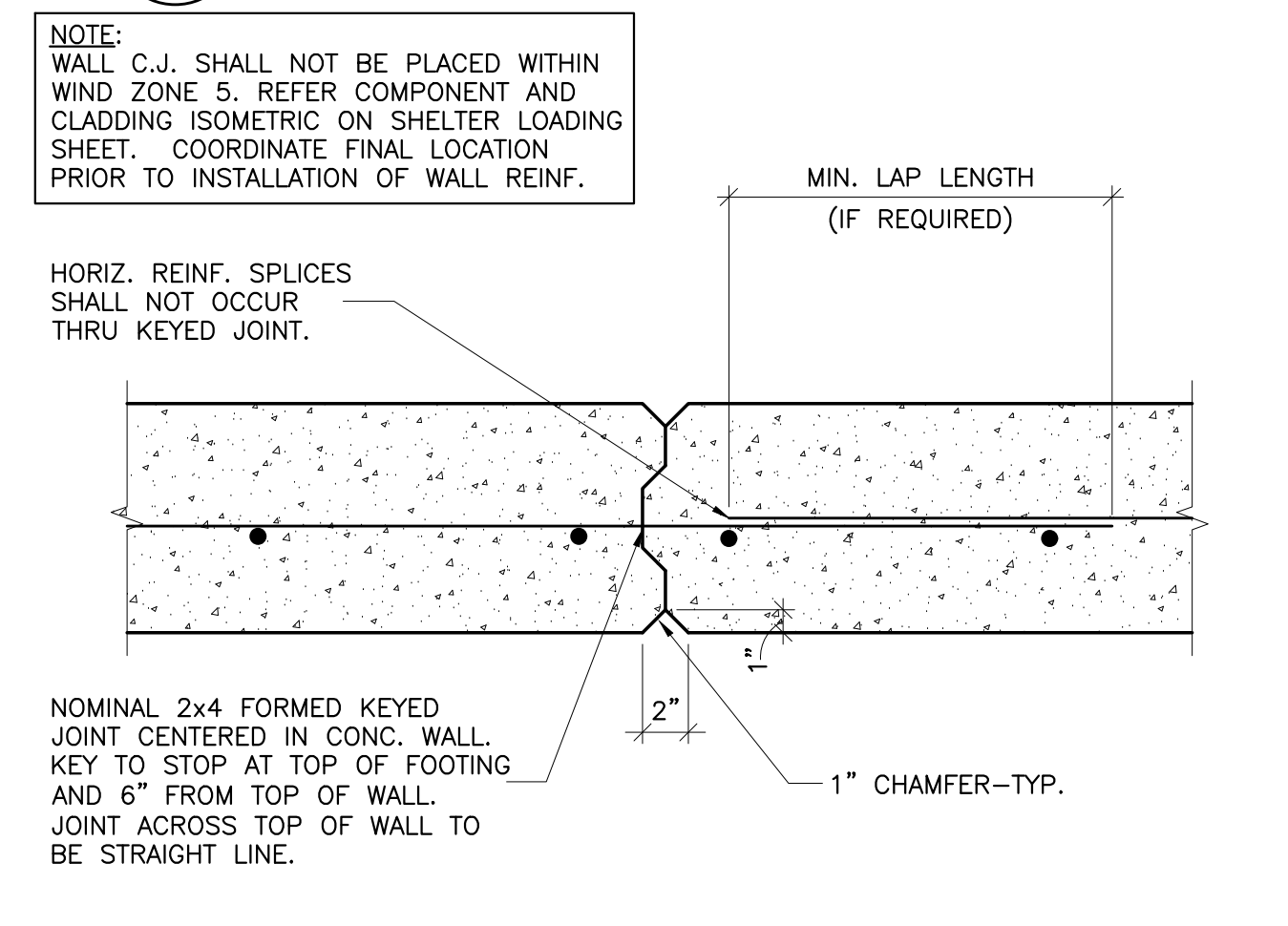
5 CJ THROUGH FOOTING
SCALE: NONE



6 TYP. SAWED JOINT (SJ)
SCALE: NONE

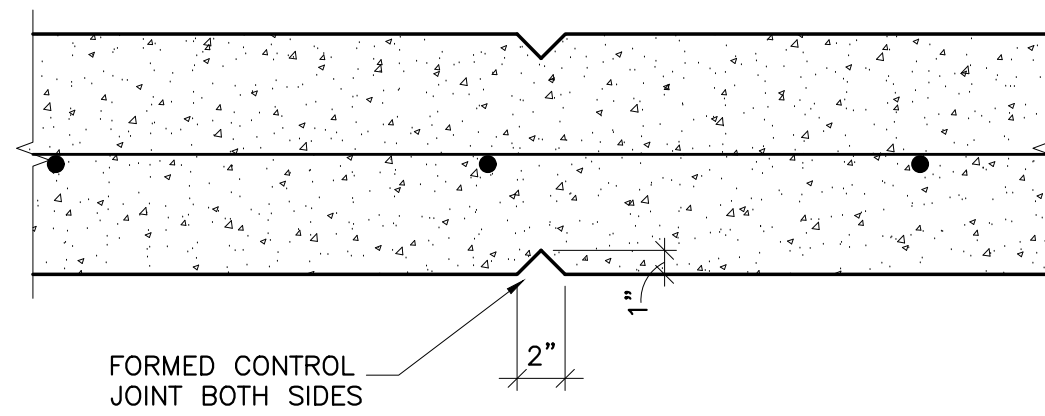


7 TYP. CONSTRUCTION JOINT (CJ)
SCALE: NONE



8 OPTIONAL VERTICAL CONSTRUCTION JOINT (C.J.)
SCALE: NONE

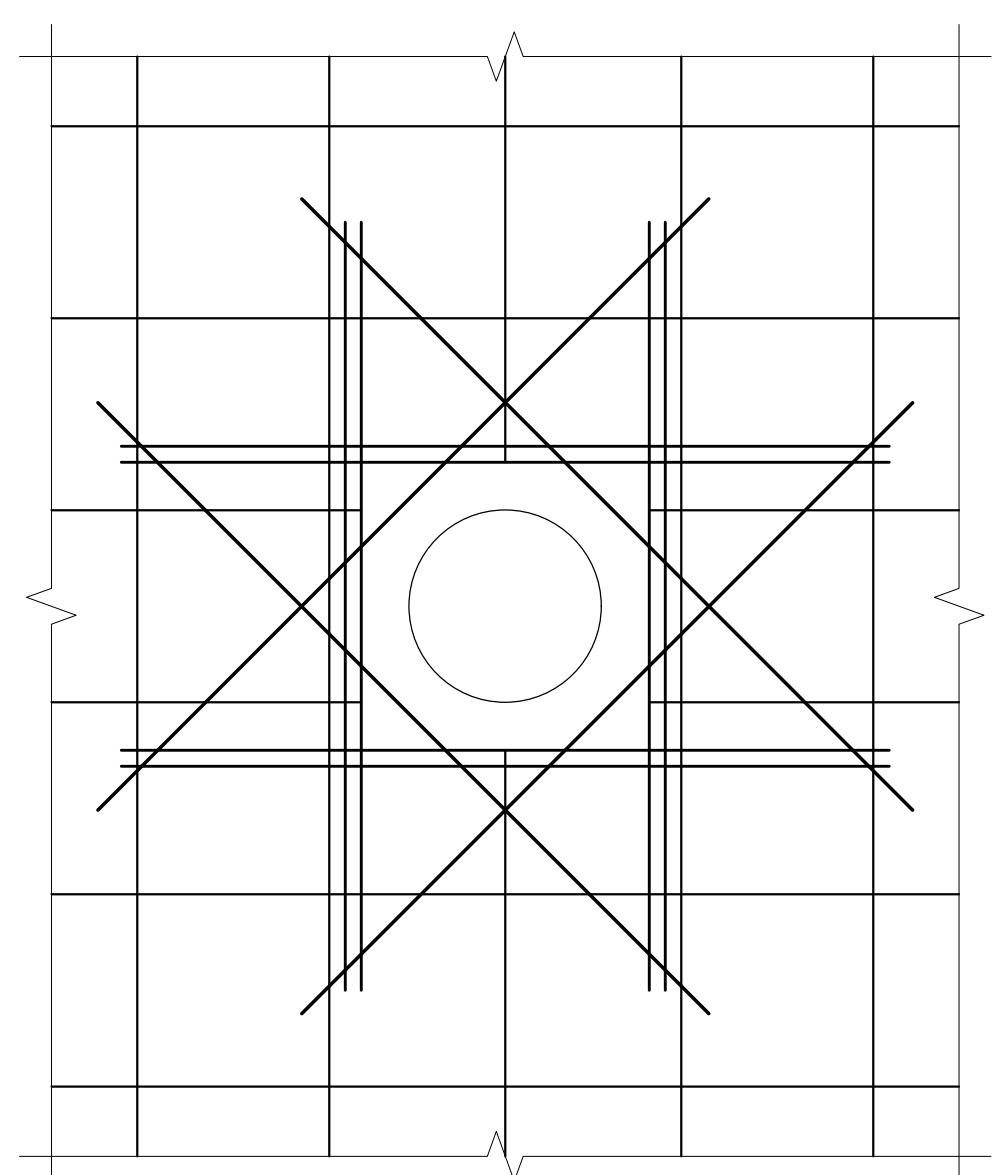
NOTES:
1. 30" MAX. SPACING TO BE COORDINATED WITH EOR AND SHOWN ON SUBMITTAL DRAWINGS.
2. WALL CONTROL JOINT SHALL NOT BE PLACED WITHIN WIND ZONE 5. REFER COMPONENT AND CLADDING ISOMETRIC ON SHELTER LOADING SHEET. COORDINATE FINAL LOCATION PRIOR TO INSTALLATION OF WALL REINF.



9 VERTICAL CONTROL JOINT
SCALE: NONE

CONCRETE EXPOSURE	MEMBER	REINFORCEMENTS	SPECIFIED COVER, IN.
CAST AGAINST AND PERMANENTLY IN CONTACT WITH GROUND	ALL	ALL	3
EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	ALL	NO. 6 THROUGH NO. 18 BAR	2
		NO. 5 BAR, W31 OR D31 WIRE, AND SMALLER	1-1/2
NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	SLAB, JOISTS, AND WALLS	NO. 14 AND NO. 18 AND SMALLER	1-1/2
		NO. 11 BAR AND SMALLER	3/4
	BEAMS, COLUMNS, PEDESTALS, AND TENSION TIES	PRIMARY REINFORCEMENT, STIRRUPS, TIES, SPIRALS, AND HOOPS	1-1/2

10 TYP. MIN. CONCRETE COVER
SCALE: NONE



11 TYP. PENETRATION THRU CONC. SLAB OR WALL
SCALE: NONE

TENSION DEVELOPMENT AND LAP-SPICE LENGTHS FOR UNCOATED REINFORCING BARS

BAR SIZE	LAP CLASS	LAP LENGTH (IN.) PER SPACING AND COVER CASE			
		f'c=3500 psi (NORMAL WEIGHT)			
		TOP BARS		OTHER BARS	
		CASE 1	CASE 2	CASE 1	CASE 2
#3	A	22	32	17	25
	B	28	42	22	32
#4	A	29	43	22	33
	B	37	56	29	43
#5	A	36	54	28	41
	B	47	70	36	54
#6	A	43	64	33	50
	B	56	84	43	64
#7	A	63	94	48	72
	B	81	122	63	94
#8	A	72	107	55	82
	B	93	139	72	107
#9	A	81	121	62	93
	B	105	157	81	121
#10	A	91	136	70	105
	B	118	177	91	136
#11	A	101	151	78	116
	B	131	196	101	151
#14	N/A	121	181	93	139
#18	N/A	161	241	124	186

NOTES: 1 in.=25.4 mm.
1. TABULATED VALUES ARE BASED ON GRADE 60 REINFORCING BARS AND NORMAL WEIGHT CONCRETE. LENGTHS ARE IN INCHES.
2. TENSION DEVELOPMENT LENGTHS AND TENSION LAP-SPICE LENGTHS ARE CALCULATED PER ACI 318, SECTIONS 12.2.2 AND 12.15, RESPECTIVELY. TABULATED VALUES FOR BEAMS OR COLUMNS ARE BASED ON TRANSVERSE REINFORCEMENT AND CONCRETE COVER MEETING MINIMUM CODE REQUIREMENTS.
3. CASES 1 AND 2, WHICH DEPEND ON THE TYPE OF STRUCTURAL ELEMENT, CONCRETE COVER, AND CENTER-TO-CENTER SPACING OF THE BARS, ARE DEFINED AS: BEAMS OR COLUMNS: CASE 1—COVER AT LEAST 1.0d AND CENTER-TO-CENTER SPACING AT LEAST 2.0d, AND CASE 2—COVER LESS THAN 1.0d OR CENTER-TO-CENTER SPACING LESS THAN 2.0d; ALL OTHERS: CASE 1—COVER AT LEAST 1.0d AND CENTER-TO-CENTER SPACING AT LEAST 3.0d, CASE 2—COVER LESS THAN 1.0d OR CENTER-TO-CENTER SPACING LESS THAN 3.0d.
4. LAP SPICE LENGTHS ARE MULTIPLES OF TENSION DEVELOPMENT LENGTHS; CLASS A=1.0d AND CLASS B=1.3d (ACI 318, SECTION 12.15.1).
5. ACI 318 DOES NOT ALLOW TENSION LAP SPICES OF #14 OR #18 BARS. THE TABULATED VALUES FOR THOSE BAR SIZES ARE THE TENSION DEVELOPMENT LENGTHS.
6. TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 IN. OF CONCRETE CAST BELOW THE BARS.
7. FOR LIGHTWEIGHT-AGGREGATE CONCRETE, MULTIPLY THE TABULATED VALUES BY 1.3.

TENSION DEVELOPMENT AND LAP-SPICE LENGTHS FOR UNCOATED REINFORCING BARS

BAR SIZE	LAP CLASS	LENGTHS (IN.) PER CONCRETE STRENGTH			
		f'c=4000 psi (NORMAL WEIGHT)			
		TOP BARS		OTHER BARS	
		CASE 1	CASE 2	CASE 1	CASE 2
#3	A	19	28	15	22
	B	24	36	19	28
#4	A	25	37	19	29
	B	32	48	25	37
#5	A	31	47	24	36
	B	40	60	31	47
#6	A	37	56	29	43
	B	48	72	37	56
#7	A	54	81	42	63
	B	70	106	54	81
#8	A	62	93	48	71
	B	80	121	62	93
#9	A	70	105	54	81
	B	91	136	70	105
#10	A	79	118	61	91
	B	102	153	79	118
#11	A	87	131	67	101
	B	113	170	87	131
#14	N/A	105	157	81	121
#18	N/A	139	209	107	161

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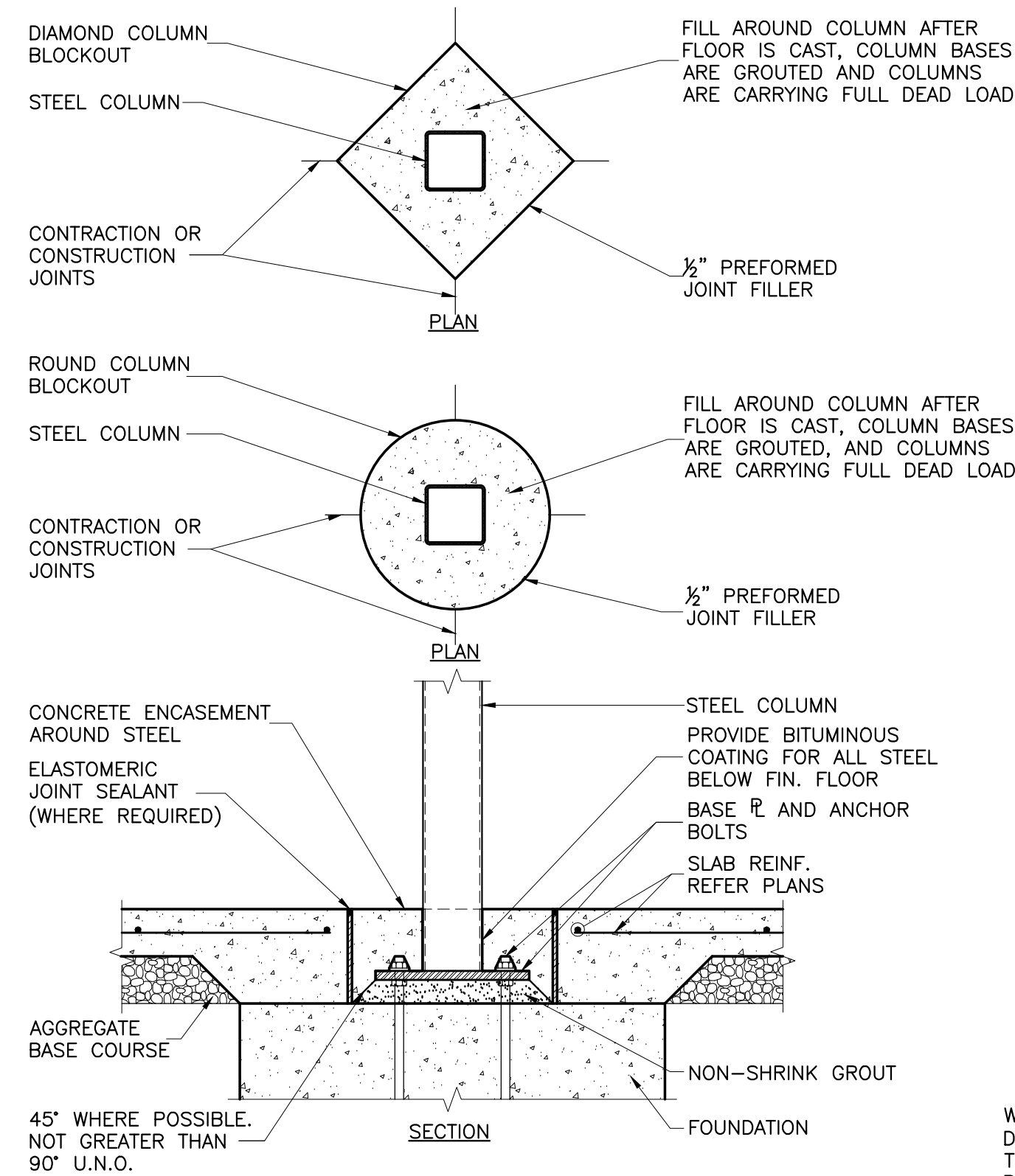
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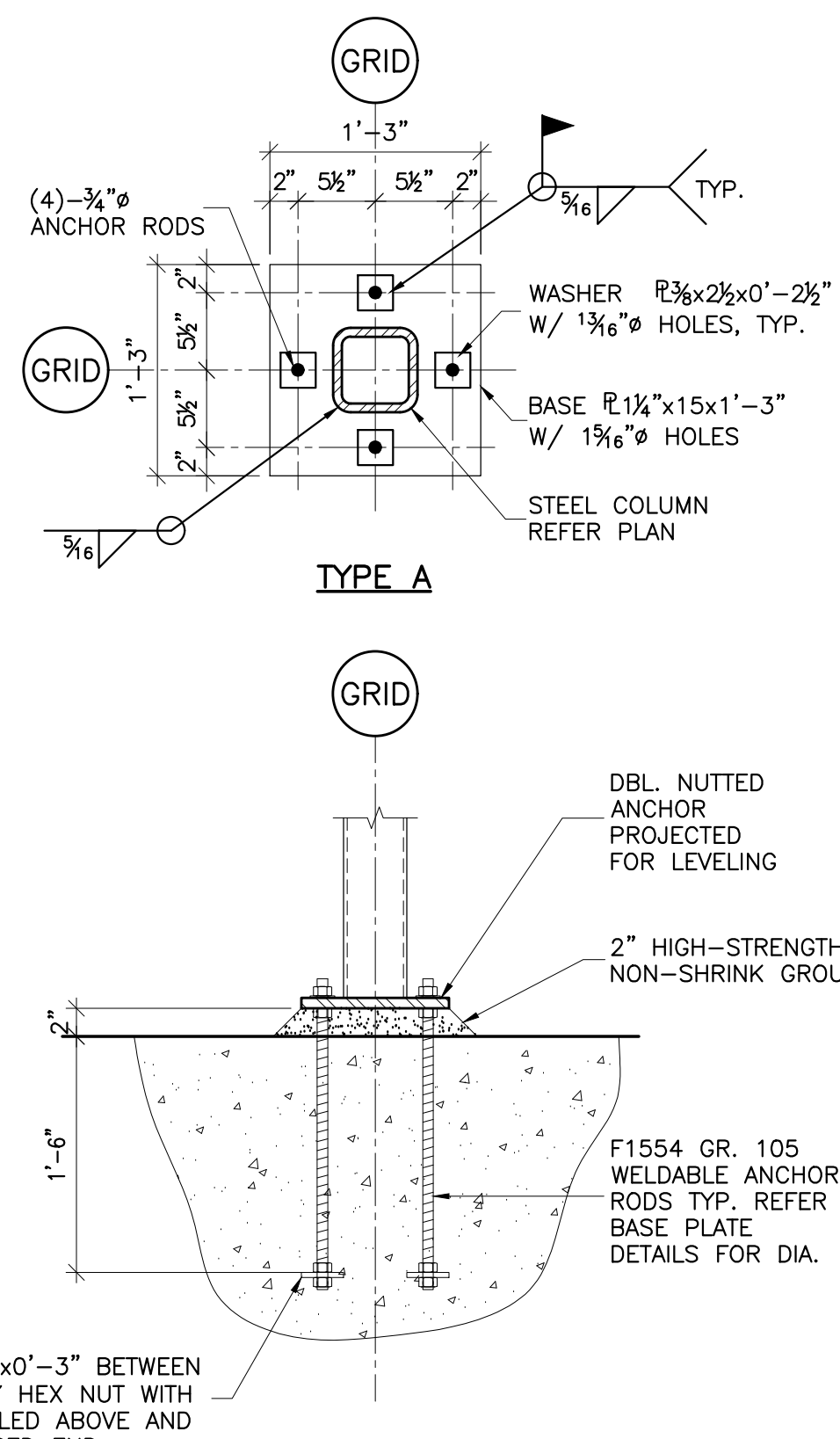
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1 TYP. ISOLATION JOINT DETAILS
SCALE: NONE

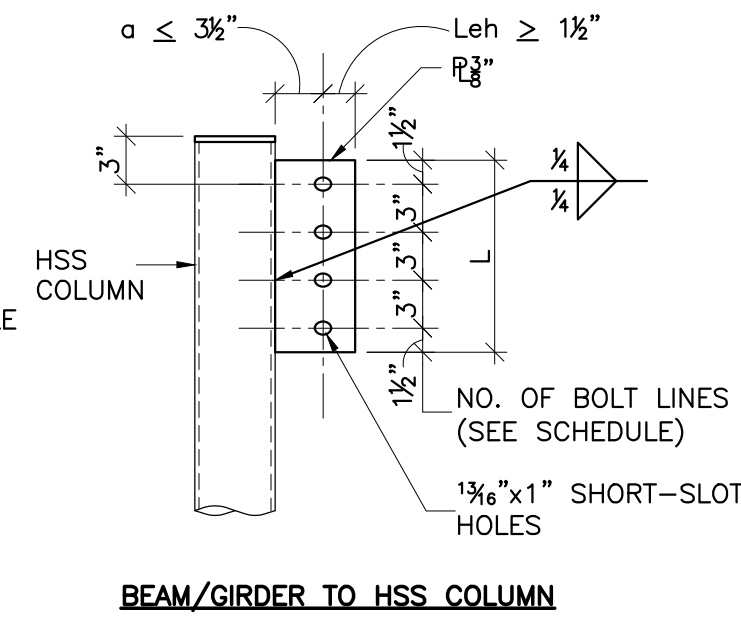
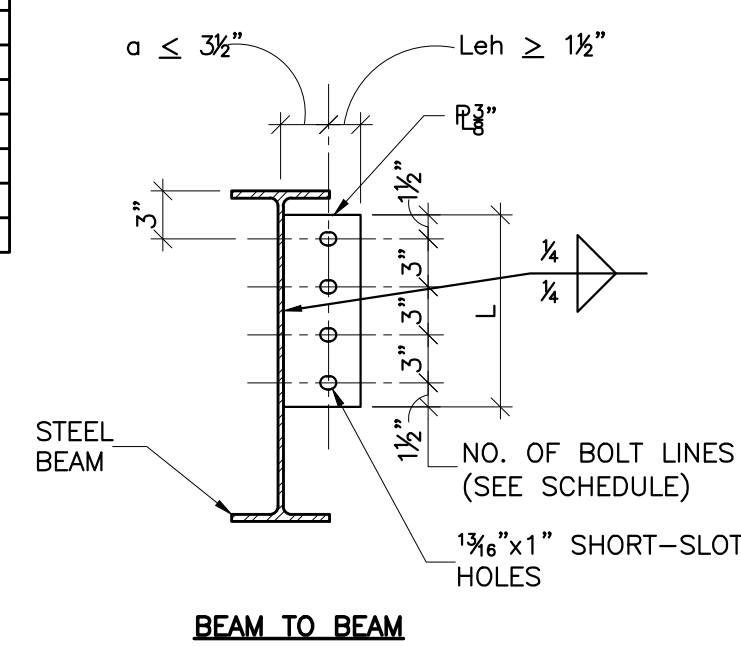


2 BASE PLATE DETAILS
SCALE: NONE

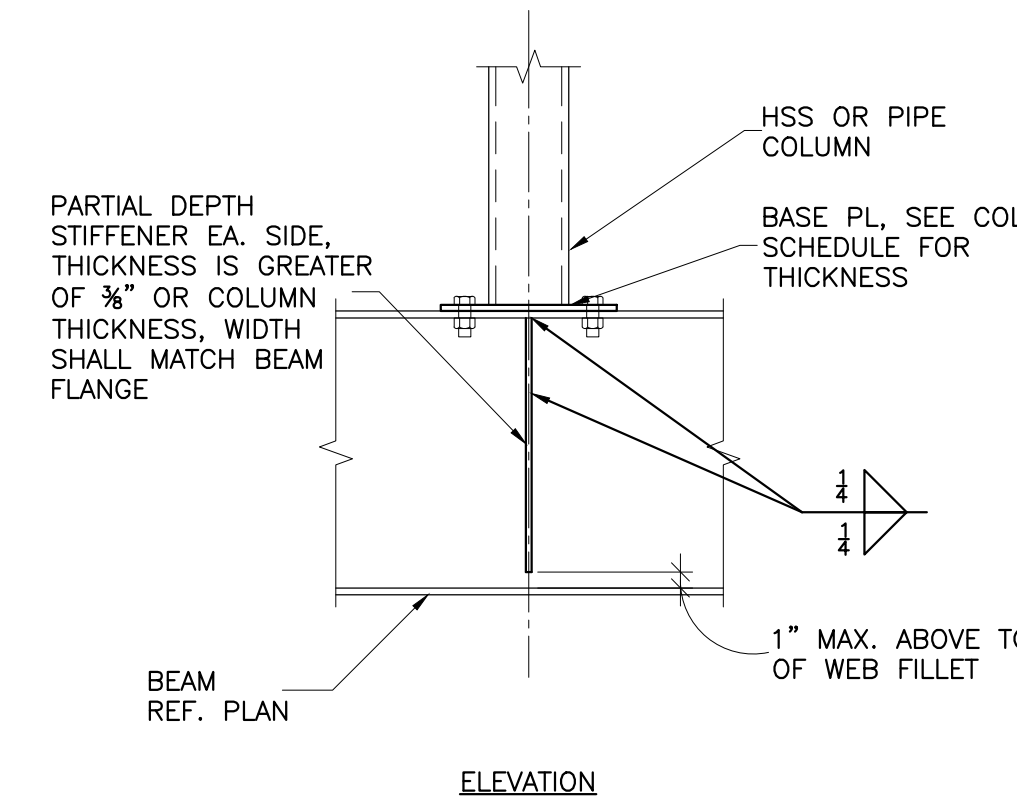
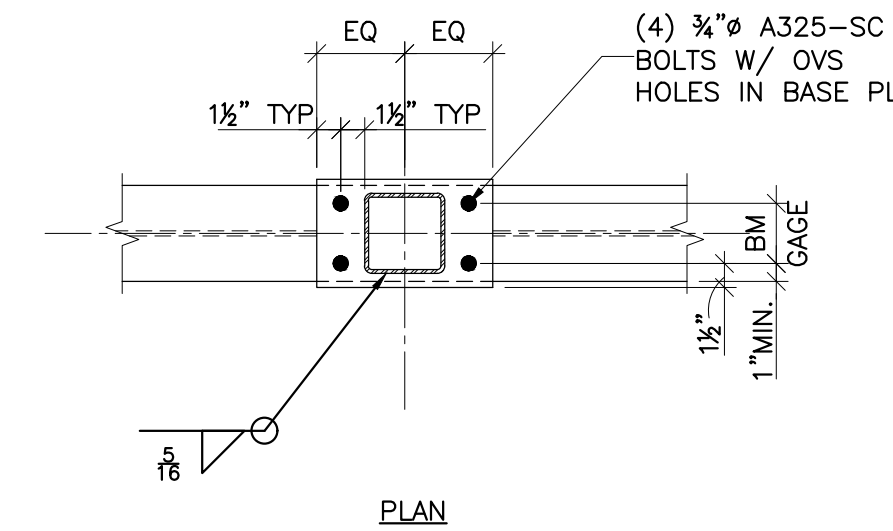
3/8\" A325-N BOLT SCHEDULE FOR SINGLE PLATE SHEAR TAB CONNECTIONS (FACTORED LOADS 14 ED. MANUAL)

END REACTION	NO. OF BOLTS
0 THRU 24 KIPS	2- ϕ
24 THRU 43 KIPS	3- ϕ
43 THRU 62 KIPS	4- ϕ
62 THRU 81 KIPS	5- ϕ
81 THRU 100 KIPS	6- ϕ
100 THRU 118 KIPS	7- ϕ
118 THRU 137 KIPS	8- ϕ

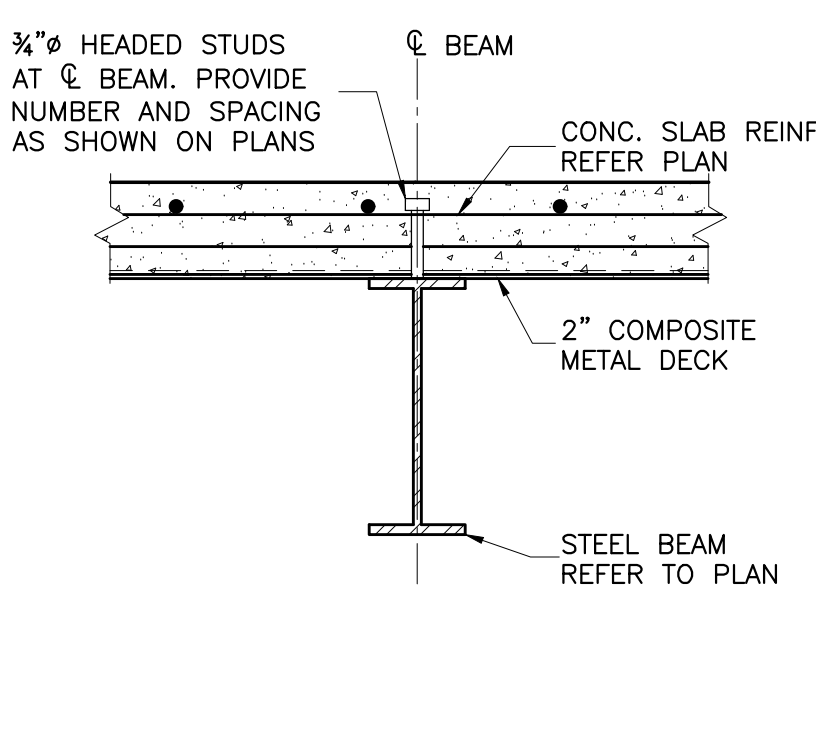
- NOTES:
- VALUES SHOWN ARE APPLICABLE FOR SINGLE PLATE SHEAR TAB CONNECTIONS
 - SEE PLAN FOR END REACTIONS
 - L = PLATE LENGTH \geq T/2 OF CONNECTED BEAM.
 - AT HSS OR PIPE COLUMNS, A THROUGH-PLATE WITH EQUAL WELD ON THE BACK SIDE OF THE COLUMN IS REQUIRED UNDER EITHER OF THE FOLLOWING CIRCUMSTANCES:
 - FOR SQUARE OR RECTANGULAR HSS:
 - WHEN $\frac{R_{\phi} - 2.7t}{0.93t} > 35.1$
 - FOR ROUND HSS OR PIPE:
 - WHEN $\frac{D}{t} > \frac{3.190}{F_y}$
 - REFER FRAMING PLANS FOR LOCATIONS WITH AXIAL TRANSFER FORCES WHICH REQUIRE MODIFIED CONNECTIONS PER DETAIL ON FRAMING ELEVATIONS SHEET.



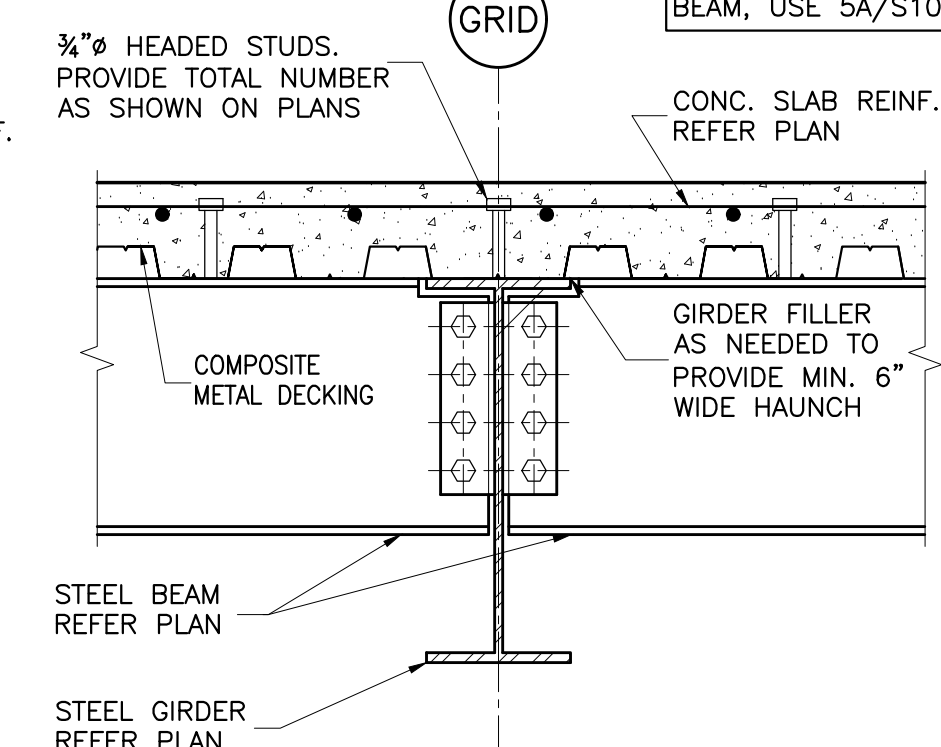
3 SINGLE PLATE SHEAR TAB CONNECTIONS (LRFD-AISC 14TH EDITION MANUAL)
SCALE: NONE



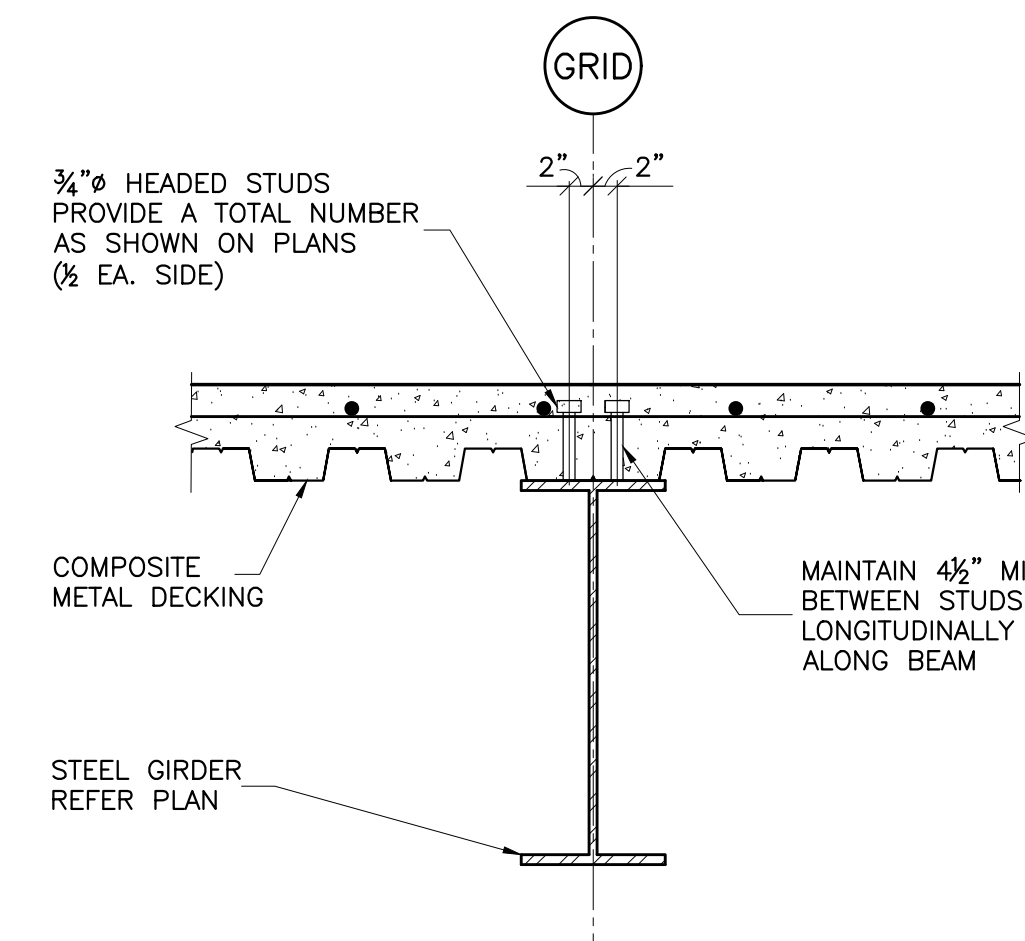
4 TYPICAL TRANSFER GIRDER DETAIL
SCALE: NONE



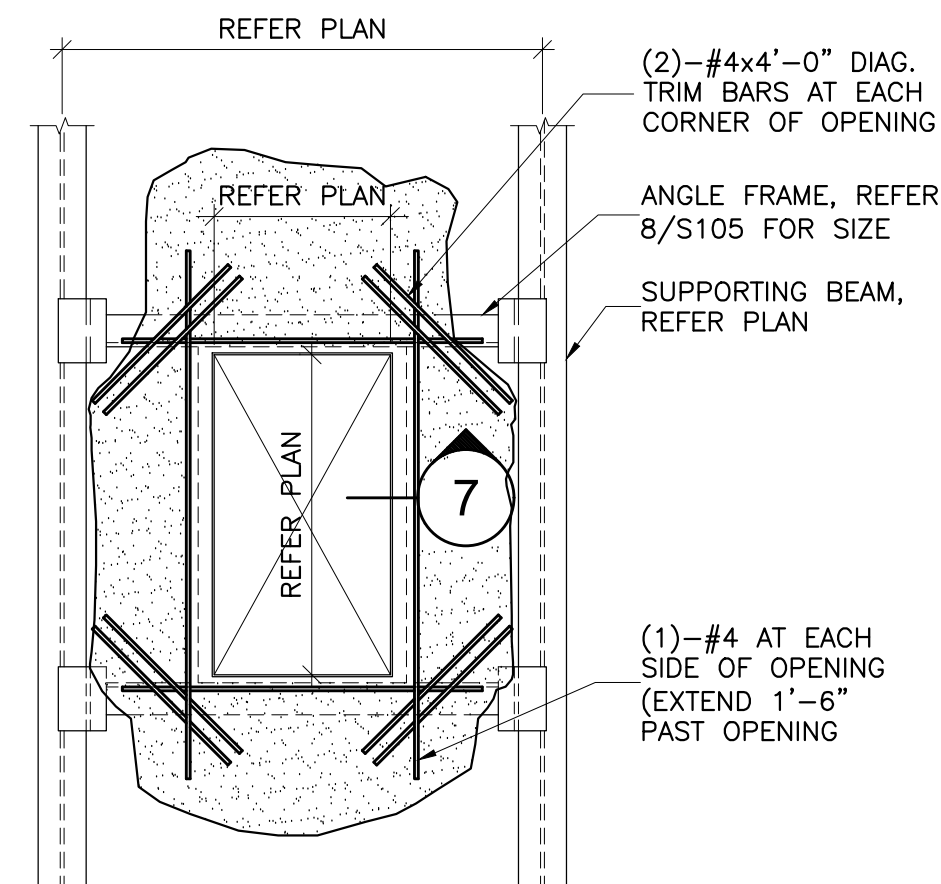
5 SECTION
SCALE: NONE



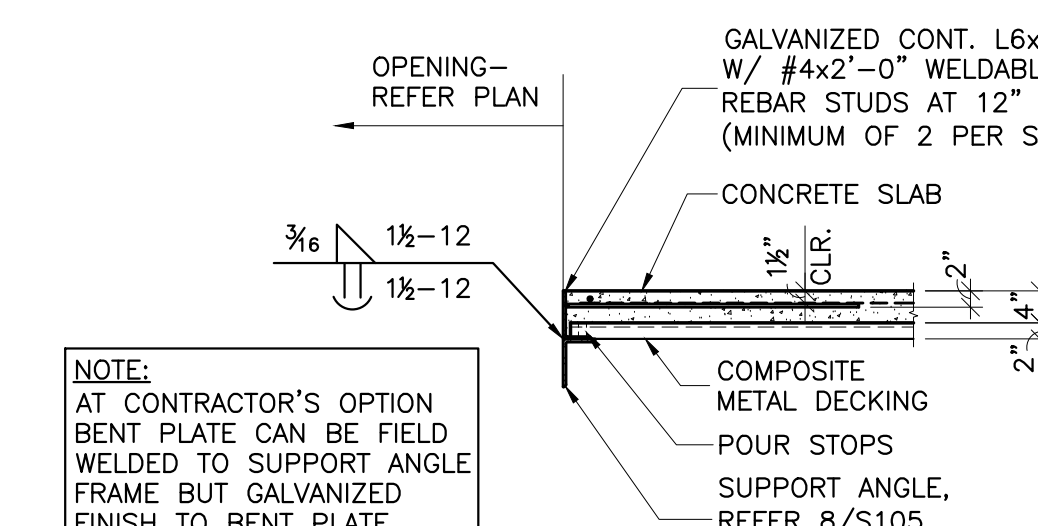
6 SECTION
SCALE: NONE



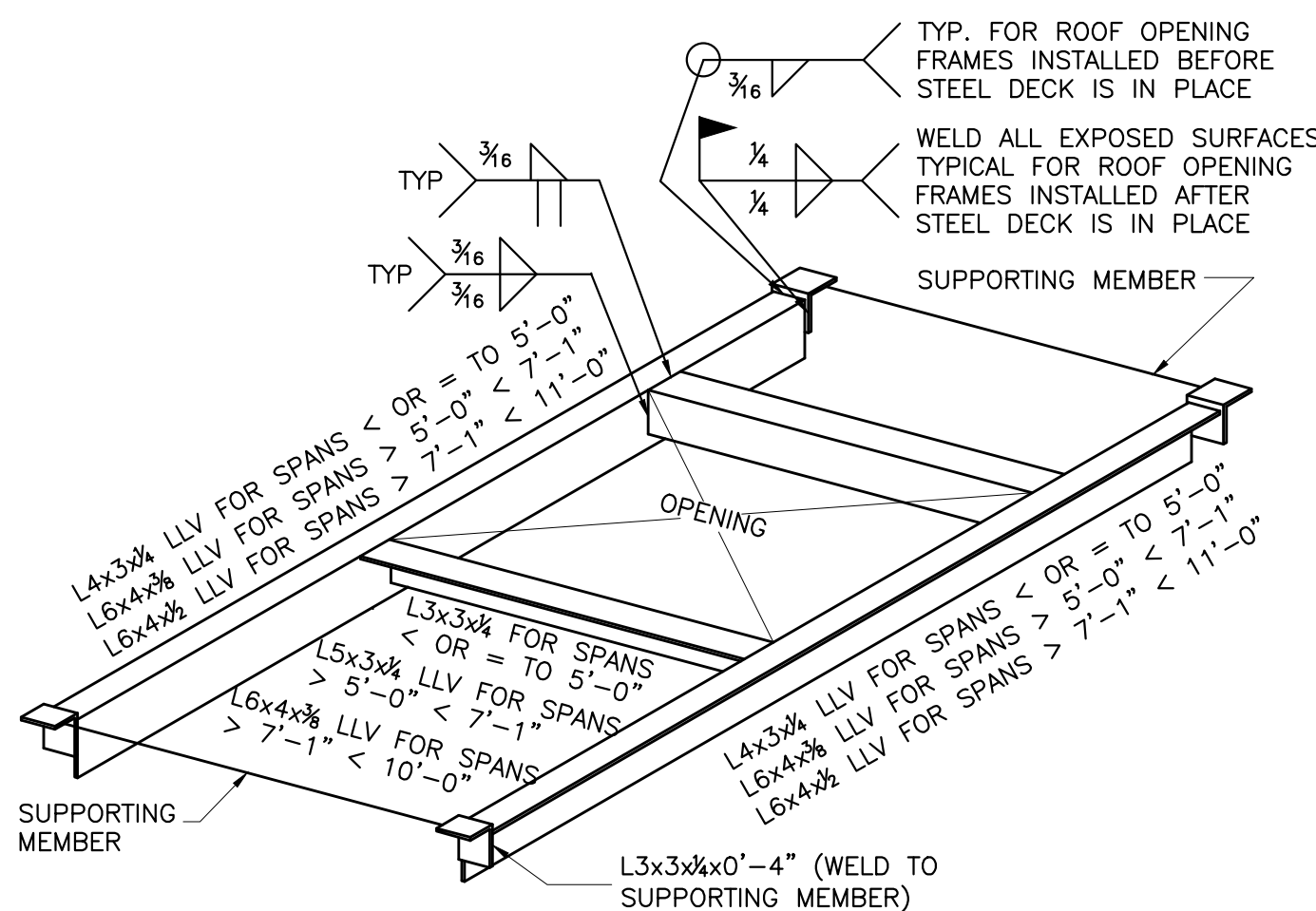
6A SECTION
SCALE: NONE



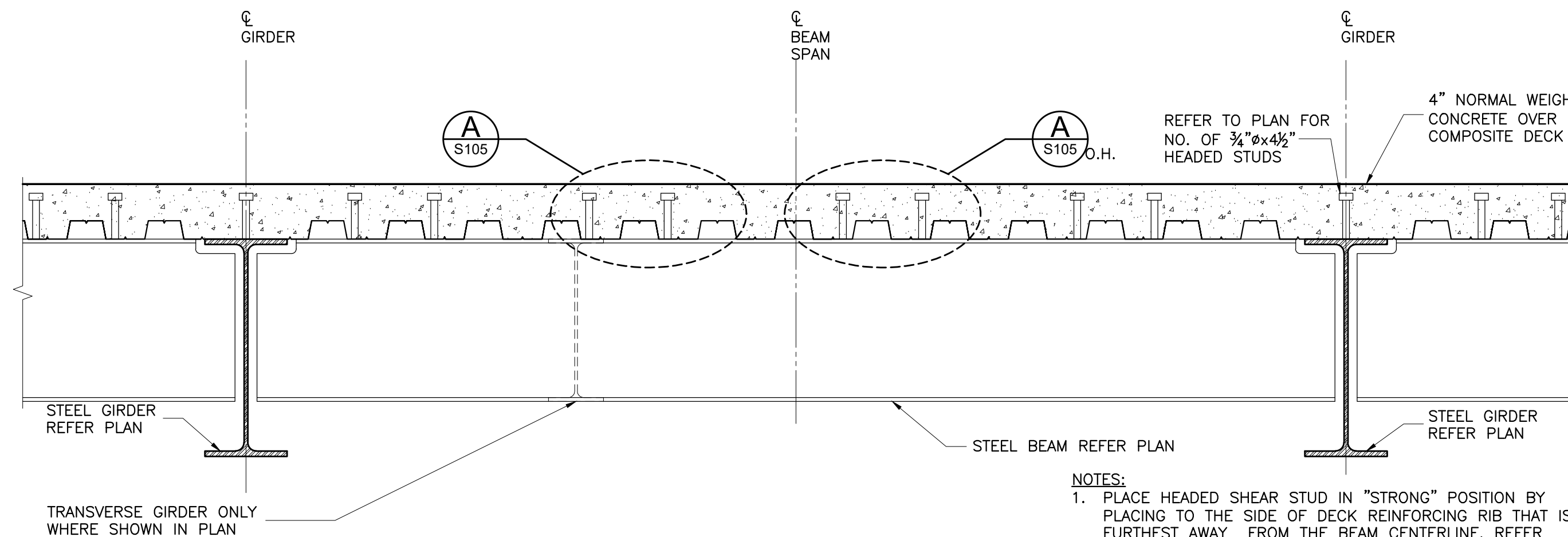
7 TYPICAL SHELTER ROOF ANGLE DETAIL
SCALE: NONE



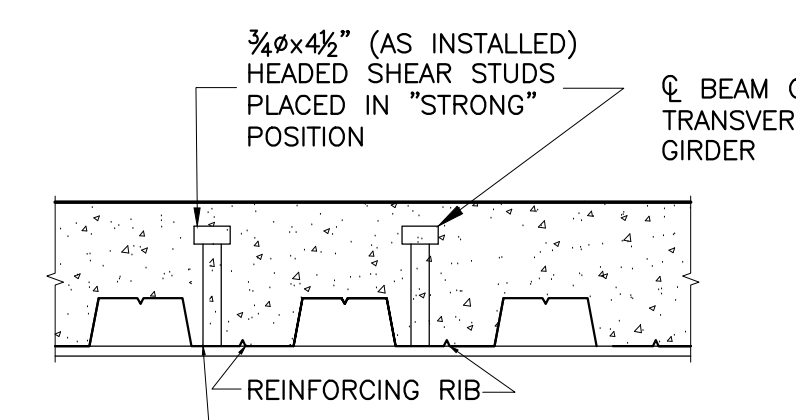
8 TYPICAL SECTION AT FLOOR OPENING
SCALE: NONE



9 TYP. ROOF OPENING FRAME AND MECHANICAL UNIT SUPPORT
SCALE: NONE



10 TYPICAL BEAM ELEVATION
SCALE: NONE



A DETAIL
SCALE: NONE

- NOTES:
- PLACE HEADED SHEAR STUD IN "STRONG" POSITION BY PLACING TO THE SIDE OF DECK REINFORCING RIB THAT IS FURTHEST AWAY FROM THE BEAM CENTERLINE. REFER DETAIL A FOR ADDITIONAL INFORMATION.
 - IN SOME CASES, TRANSVERSE GIRDERS FRAME INTO THE BEAM NOT AT CENTERLINE, BUT WITHIN A FEW FEET. AT THIS CONDITION, PLACE STUDS IN "STRONG" POSITION RELATIVE TO TRANSVERSE GIRDER AND NOT CENTERLINE OF BEAM.



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REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION			
VERIFICATION AND INSPECTION	FREQUENCY OF INSPECTION		REFERENCED STANDARD
	CONTINUOUS (inspect each joint/member)	PERIODIC (inspect random joint/members)	
1. Material verification of high-strength bolts, nuts and washers:			
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	-	QC and QA	AISC 360, Section A3.3 and applicable ASTM material standards
b. Manufacturer's certifications available for fastener materials.	QA	QC	AISC 360, Table N5.6-1
c. Fasteners marked in accordance with ASTM requirements.	-	QC and QA	
d. Proper fasteners selected for the joint detail (grade, type, bolt length if threads are to be excluded from shear plane).	-	QC and QA	
e. Proper bolting procedure selected for joint detail.	-	QC and QA	
f. Connecting elements, including the appropriate faying surface condition and hole preparation, if specified, meet applicable requirements.	-	QC and QA	AISC 360, Table N5.4-2 During Welding
g. Pre-installation verification testing by installation personnel observed and documented for fastener assemblies and methods used.	QC	QA	
h. Proper storage provided for bolts, nuts, washers and other fastener components.	-	QC and QA	
2. Inspection of high-strength bolting:			
<ul style="list-style-type: none"> For bolts requiring pretensioning, the special inspector shall observe the preinstallation testing and calibration procedures; determine that all piles of connected materials have been drawn together and properly snugged prior to pretensioning and monitor the installation of bolts to verify that fasteners are pretensioned in accordance with the RCSC Specification, progressing systematically from the most rigid point to the free edges. For joints required to be tightened only to the snug-tight condition, the special inspector need only verify that the connected materials have been drawn together and properly snugged. 	-	QC and QA	AISC 360, Section M2.5
a. Snug-tight joints.	-	QC and QA	
b. Pretensioned and slip-critical joints using turn-of-nut with matchmarking, twist-off bolt or direct tension indicator methods of installation.	-	QC and QA	
c. Pretensioned and slip-critical joints using turn-of-nut without matchmarking of calibrated wrench methods of installation.	QC and QA	-	
d. Fastener assemblies, of suitable condition, placed in all holes and washers (if required) are positioned as required.	-	QC and QA	
e. Fastener component not turned by the wrench prevented from rotating.	-	QC and QA	
f. Document acceptance or rejection of bolted connections.	QC and QA	-	AISC 360, Table N5.6-2
3. Material verification of structural steel and cold-formed steel deck U.N.O.:			
a. For structural steel, identification markings to conform to AISC 360.	-	QC and QA	AISC 360, Section M1
b. For other steel, identification markings to conform to ASTM standards specified in the approved construction documents.	-	QC and QA	Applicable ASTM material standards
4. Inspection prior to welding:			
a. Verify identification markings of weld filler materials conform to AWS specification in the approved construction documents.	-	QC and QA	AISC 360, Section A3.5 and applicable AWS AS documents
b. Welding procedure specifications are available.	QC and QA	-	AISC 360, Table N5.4-1
c. Manufacturer certifications for welding consumables available.	QC and QA	-	
d. Material identification (type/grade) and welded identification system.	-	QC and QA	
e. Fit-up of welds including but not limited to joint preparation, dimensions, cleanliness, tacking, and backing type/fit as applicable.	-	QC and QA	
f. Configuration and finish of access holes	-	QC and QA	
g. Check welding equipment.	-	QC	

REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION			
VERIFICATION AND INSPECTION	FREQUENCY OF INSPECTION		REFERENCED STANDARD
	CONTINUOUS (inspect each joint/member)	PERIODIC (inspect random joint/members)	
5. Inspection of welding:			
a. AISC 360 requirements for welding structural steel			
1) Use of qualified welders	-	QC and QA	AISC 360, Table N5.4-2 During Welding
2) Packaging and exposure control and handling of welding consumables.	-	QC and QA	
3) Welding over cracked tack welds	-	QC and QA	
4) Environmental conditions including but not limited to precipitation, temperature and wind.	-	QC and QA	
5) Verify settings on equipment, travel speeds, electrode materials, shielding gas type/flow rate, preheating interpass temperatures and proper position meets WPS standards.	-	QC and QA	AISC 360, Table N5.4-2 After Welding
6) Verify welding techniques for interpass, final cleaning, profile limitations, and quality requirements.	-	QC and QA	
7) Welds are cleaned and painted where required.	-	QC and QA	
8) Verify size, length and locations of welds.	QC and QA	-	AISC 360, Table N5.4-2 After Welding
9) Visually verify welds for crack prohibition, weld/base-metal fusion, crater cross section, weld profiles, weld size, undercutting, and porosity.	QC and QA	-	
10) Arc strikes, k-area cracks within 3" of weld, removal of backing, and repair activities as applicable.	QC and QA	-	
11) Documentation of acceptance or rejection of welded joint or member.	QC and QA	-	
b. American Welding Society requirements for structural steel and cold-formed steel deck:			
1) Complete and partial joint penetration groove welds.	X	-	AWS D1.1
2) Multipass fillet welds.	X	-	
3) Single-pass fillet welds > 5/16"	X	-	
4) Plug and slot welds.	X	-	
5) Single-pass fillet welds ≤ 5/16"	-	X	
6) Floor and roof deck welds.	-	X	AWS D1.3
7) Welded studs & deformed bar anchors (DBA's).	-	X	AWS D1.1
8) Welded sheet steel for cold-formed steel members	-	X	AWS D1.3
9) Welding of stairs & railing systems	-	X	AWS D1.1
c. Reinforcing steel:			
1) Verification of weldability of reinforcing steel other than ASTM A 706.	-	X	AWS D1.4, ACI 318: Section 3.5.2
2) Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.	X	-	
3) Shear reinforcement.	X	-	
4) Other reinforcing steel.	-	X	
6. Inspection of steel elements of composite construction prior to concrete placement:			
a. Placement and installation of steel deck.	QC and QA	-	AISC 360, Table N6.1
b. Placement and installation of steel HSA.	QC and QA	-	AISC 360, Table N6.1
c. Documentation of acceptance or rejection of steel elements.	QC and QA	-	AISC 360, Table N6.1

TABLE 1705.3 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION				
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD	IBC REFERENCE
1) Inspect reinforcement, including prestressing tendon, and verify placement.	-	X	ACI 318 Ch. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2) Reinforcing bar welding: a) Verify weldability of reinforcing bars other than ASTM A706; b) Inspect single-pass fillet welds, maximum 5/16"; and c) Inspect all other welds.	-	X	AWS D1.4 ACI 318: 26.6.4	-
3) Inspect anchors cast in concrete.	-	X	ACI 318:17.8.2	-
4) Inspect anchors post-installed in hardened concrete members: a) Adhesive anchors installed in horizontally or upwardly inclined orientations to resist sustained tension loads. b) Mechanical anchors and adhesive anchors not defined in 4.a.	X	-	ACI 318: 17.8.2.4	-
5) Verify use of required design mix.	-	X	ACI 318: Ch. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6) Prior to concrete placement, fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	-	ASTM C 172 ASTM C 31 ACI 318: 26.5, 26.12	1908.10
7) Inspect concrete and shotcrete placement for proper application techniques.	X	-	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8) Verify maintenance of specified curing temperature and techniques.	-	X	ACI 318: 26.5.3-26.5.5	1908.9
9) Inspect prestressed concrete for: a) Application of prestressing forces; and b) Grouting of bonded prestressing tendons.	X	-	ACI 318: 26.10	-
10) Inspect erection of precast concrete members.	-	X	ACI 318: Ch. 26.9	-
11) Verify in-situ concrete strength, prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	-	X	ACI 318: 26.11.2	-
12) Inspect formwork for shape, location and dimensions of the concrete member being formed.	-	X	ACI 318: 26.11.1.2(b)	-

TABLE 1705.6 REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS		
TYPE	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	-	X
2. Verify excavations are extended to proper depth and have reached proper material.	-	X
3. Perform classification and testing of compacted fill materials.	-	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X	-
5. Prior to placement of compacted fill, inspect subgrade and verify that site has been prepared properly.	-	X

1. STATEMENT OF SPECIAL INSPECTIONS NOTES:

- A. THIS STATEMENT OF SPECIAL INSPECTIONS IS INCLUDED AS REQUIRED BY SECTIONS 1704.1 AND 1705 OF THE 2018 INTERNATIONAL BUILDING CODE.
- B. SPECIAL INSPECTIONS SHALL CONFORM TO CHAPTER 17 OF THE 2018 INTERNATIONAL BUILDING CODE AND AS SUMMARIZED HEREIN. GENERAL REQUIREMENTS ARE LISTED BELOW AND IN THE ATTACHED INSPECTION TABLES.
- C. REFER TO PROJECT SPECIFICATIONS FOR ADDITIONAL SPECIAL INSPECTION REQUIREMENTS. IF CONFLICTING REQUIREMENTS ARE FOUND BETWEEN THIS STATEMENT OF SPECIAL INSPECTIONS AND THE PROJECT SPECIFICATIONS, THE MORE STRINGENT PROVISION SHALL CONTROL UNLESS DIRECTED OTHERWISE IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD.
- D. THE GENERAL CONTRACTOR SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS FOR THIS PROJECT. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR THE INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.
- E. THE SPECIAL INSPECTOR SHALL PROVIDE WRITTEN DOCUMENTATION TO THE BUILDING OFFICIAL DEMONSTRATING HIS OR HER COMPETENCE AND RELEVANT EXPERIENCE OR TRAINING. EXPERIENCE OR TRAINING SHALL BE CONSIDERED RELEVANT WHEN THE DOCUMENTED EXPERIENCE OR TRAINING IS RELATED IN COMPLEXITY TO THE SAME TYPE OF SPECIAL INSPECTION ACTIVITIES FOR PROJECTS OF SIMILAR COMPLEXITY AND MATERIAL QUALITIES.
- F. THE SPECIAL INSPECTOR SHALL PROVIDE CONTINUOUS OR PERIODIC INSPECTIONS AS SHOWN IN THE ATTACHED INSPECTION TABLES
- 1) CONTINUOUS INSPECTION: THE SPECIAL INSPECTOR SHALL BE PRESENT AT ALL PROCEDURAL EVENTS.
- 2) PERIODIC INSPECTION: THE SPECIAL INSPECTOR SHALL BE PRESENT AT THE START OF THE WORK AND PERIODIC INSPECTION IS MADE TO VERIFY PROGRESS OF WORK IS IN COMPLIANCE.
- G. INSPECTION OF FABRICATORS: WHERE FABRICATION OF STRUCTURAL LOADBEARING MEMBERS AND ASSEMBLIES IS BEING PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP, SPECIAL INSPECTION OF THE FABRICATED ITEMS SHALL BE REQUIRED BY SECTION 1704.2.5 OF THE 2018 INTERNATIONAL BUILDING CODE AND AS REQUIRED ELSEWHERE IN THE CODE.
- H. FABRICATOR APPROVAL: SPECIAL INSPECTIONS REQUIRED BY SECTION 1704 ARE NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION. APPROVAL SHALL BE BASED UPON REVIEW OF THE FABRICATOR'S WRITTEN PROCEDURAL AND QUALITY CONTROL MANUALS AND PERIODIC AUDITING OF FABRICATION PRACTICES BY AN APPROVED SPECIAL INSPECTION AGENCY. AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL STATING THAT THE WORK WAS PERFORMED IN ACCORDANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.
- I. REPORT REQUIREMENTS: SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE. REPORTS SHALL INDICATE THAT WORK INSPECTED WAS OR WAS NOT COMPLETED IN CONFORMANCE TO APPROVED CONSTRUCTION DOCUMENTS. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THEY ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND TO THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE PRIOR TO THE COMPLETION OF THAT PHASE OF THE WORK. A FINAL REPORT DOCUMENTING REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED AT A POINT IN TIME AGREED UPON PRIOR TO THE START OF WORK BY THE APPLICANT AND THE BUILDING OFFICIAL.
- J. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING REASONABLE NOTICE TO THE SPECIAL INSPECTOR(S) REGARDING WHEN ELEMENTS OF THE PROJECT WILL BE READY FOR EFFICIENT IMPLEMENTATION OF SPECIAL INSPECTIONS.
- K. THE CONTRACTOR SHALL PROVIDE ACCESS TO THE LATEST VERSION OF ALL APPROVED PLANS AND SHOP DRAWINGS FOR THE SPECIAL INSPECTOR'S USE IN PERFORMING SPECIAL INSPECTIONS.
- L. CONTRACTOR SHALL GRANT ACCESS TO OWNER'S SPECIAL INSPECTOR AS IS REASONABLY NECESSARY FOR THE PROPER PERFORMANCE OF SPECIAL INSPECTIONS.
- M. SPECIAL INSPECTIONS DO NOT RELIEVE THE CONTRACTOR OF RESPONSIBILITY TO COMPLY WITH ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS. CONSTRUCTION MEANS AND METHODS AND JOBSITE SAFETY ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

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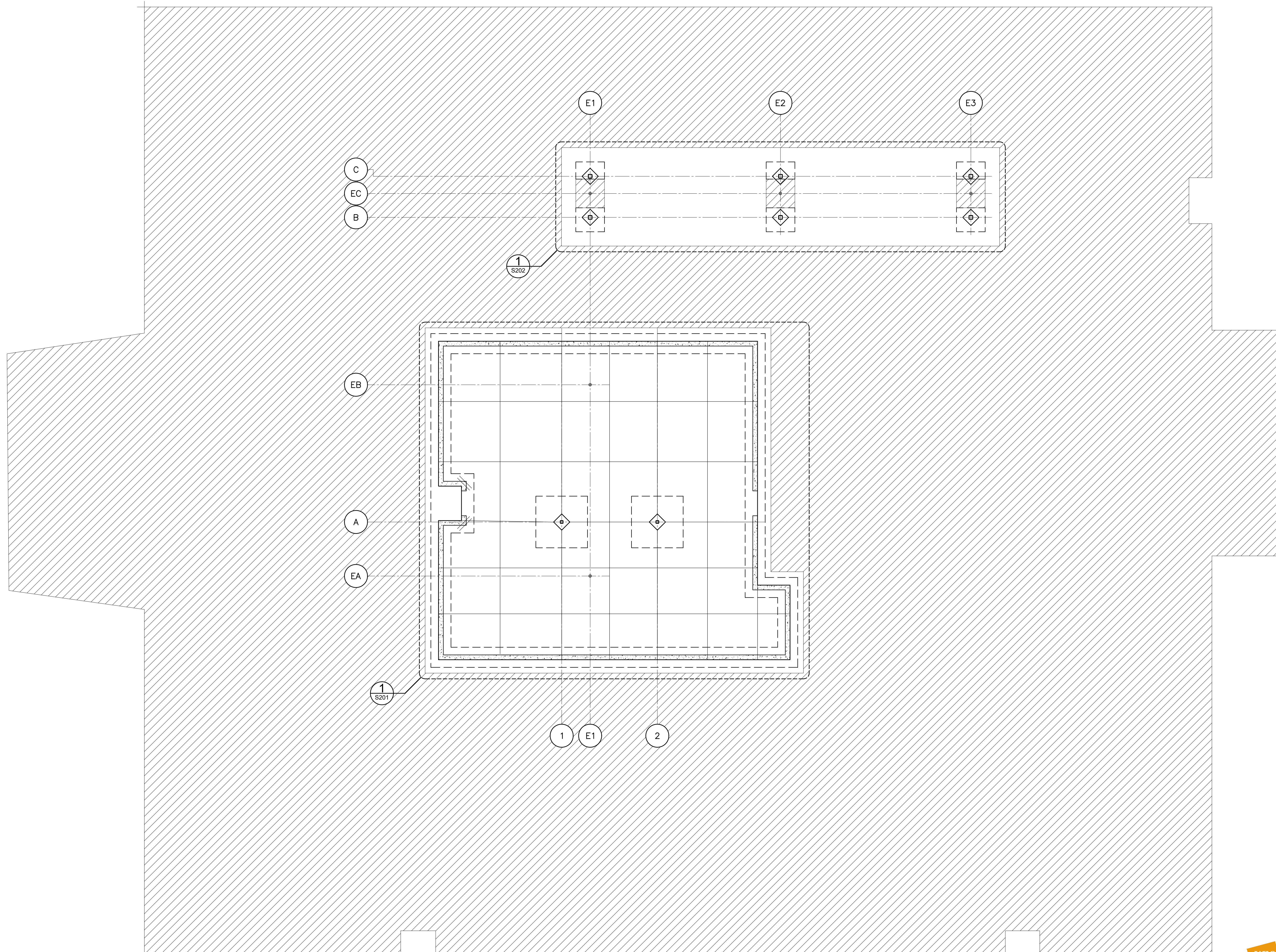
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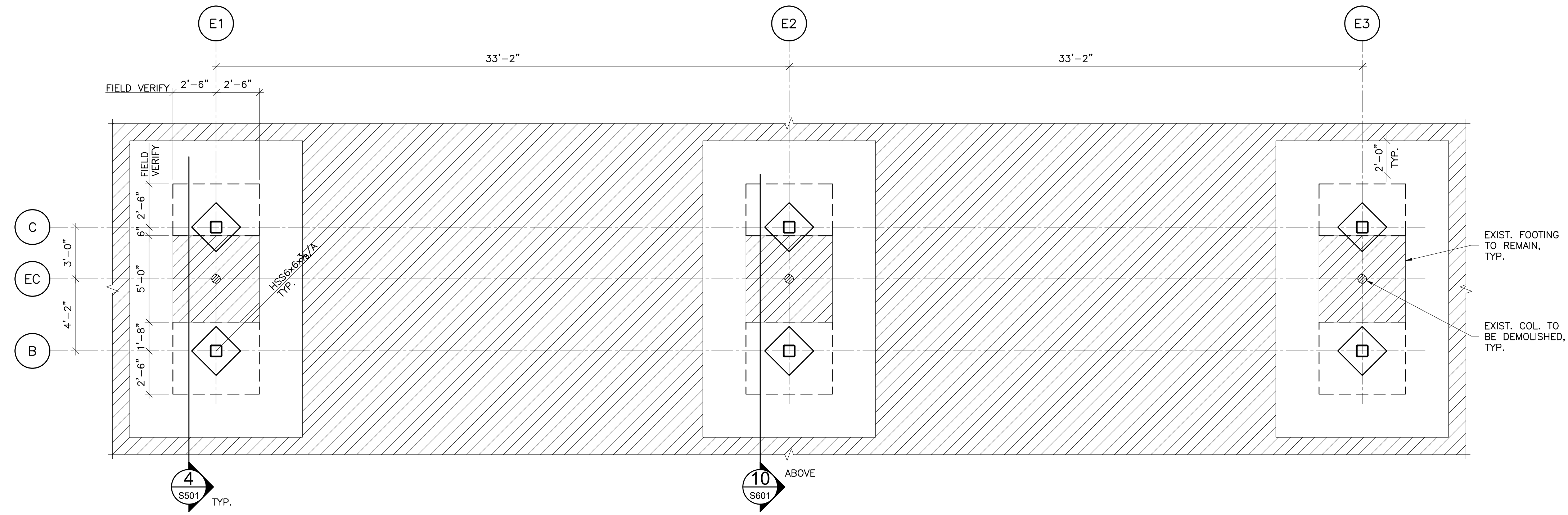
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OVERALL FOUNDATION PLAN

SCALE: 1/8"=1'-0"



 **1** PARTIAL FOUNDATION PLAN
S202 SCALE: 1/4"=1'-0"

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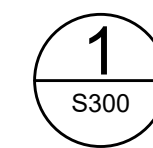
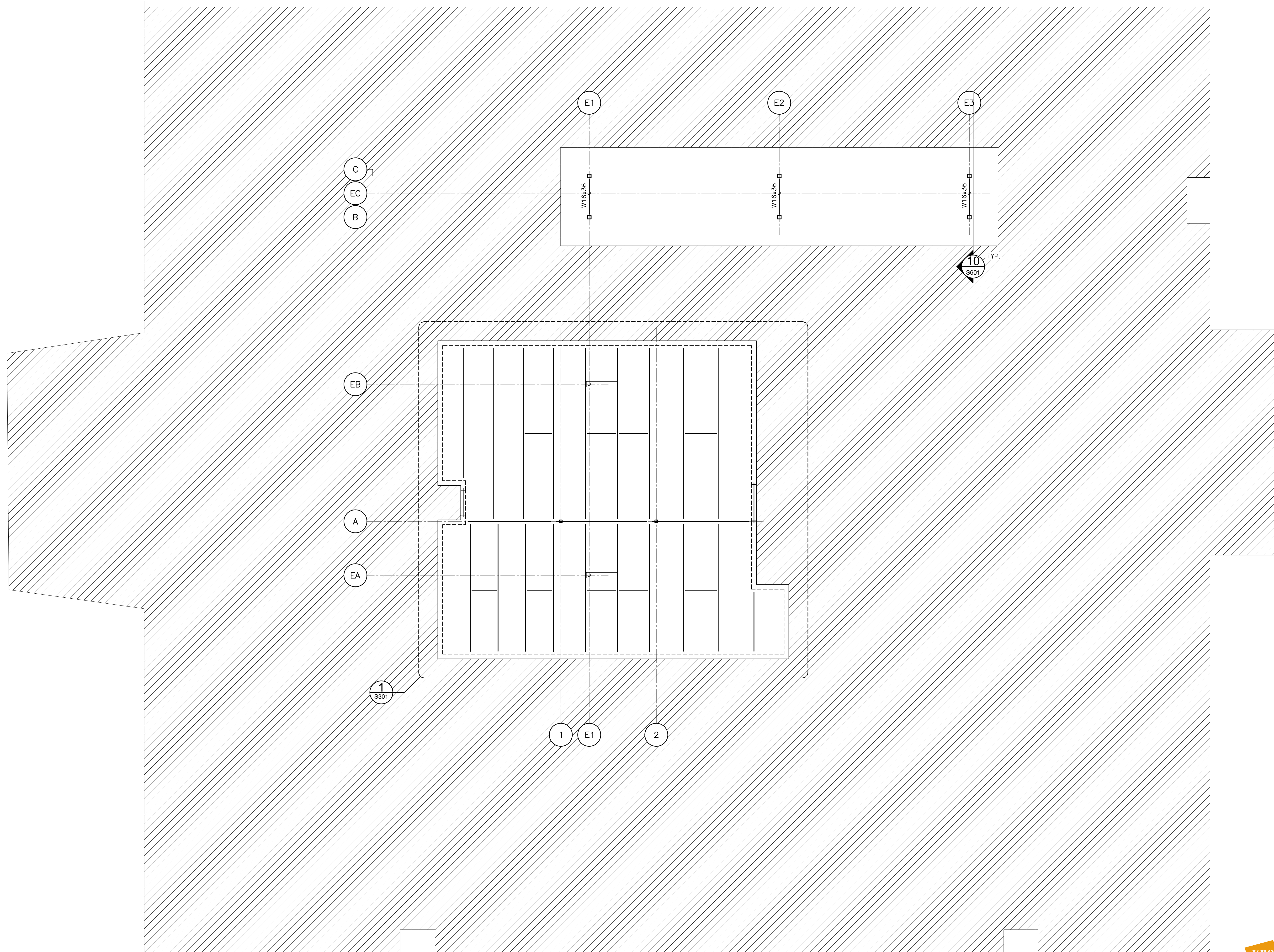
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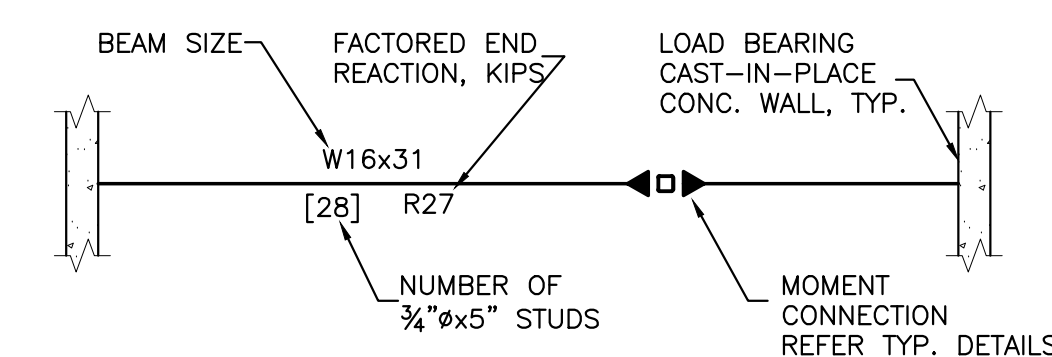
1 OVERALL FRAMING PLAN

S300 SCALE: 1/8"=1'-0"



- SHELTER ROOF FRAMING PLAN NOTES:**
1. ALL ELEVATIONS ARE REFERENCED FROM FINISHED FLOOR DATUM OF 100'-0". REFER GENERAL NOTES FOR ACTUAL ELEVATION.
 2. [10] INDICATES THE NUMBER OF 3/4"x5" HEADED STUDS THAT ARE REQUIRED. AT BEAMS, HEADED STUDS ARE UNIFORMLY SPACED ALONG BEAM LENGTH. AT GIRDERS, STUDS ARE UNIFORMLY SPACED BETWEEN INTERSECTING TRANSVERSE BEAMS. LENGTH OF STUD IS THE FINAL INSTALLED LENGTH AFTER WELDING. SELECT LENGTH OF STUD PRIOR TO WELDING BASED ON BURN THROUGH CONDITIONS, I.E., THROUGH METAL DECK OR DIRECTLY TO STEEL.
 3. AT COMPOSITE BEAMS, PLACE HEADED STUDS IN THE "STRONG" POSITION. REFER TYPICAL DETAILS.
 4. PROVIDE GIRDER FILLERS TO PROVIDE AT LEAST A 6" HAUNCH DIMENSION AT GIRDERS. REFER TYPICAL DETAILS.
 5. PROVIDE 16 GAGE SHEET METAL CLOSURES AT COLUMN TO BEAM OR GIRDER CONNECTIONS PRIOR TO CONCRETE PLACEMENT.
 6. PROVIDE [2] #4x4'-0" DIAGONAL BARS AT ALL RE ENTRANT CORNERS.
 7. ALL CONCRETE SLABS SHALL BE WET CURED FOR A MINIMUM OF 7 DAYS. USE OF SPRAY-ON OR ROLL-ON CURING COMPOUND IS PROHIBITED.
 8. PROVIDE CONTINUOUS BUTT SPLICE WELDING IN FIELD AT DECK ANGLES.
 9. FOR COLUMN SIZES, REFER TO FOUNDATION PLAN
 10. ALL ROOF OPENINGS FOR MECHANICAL ROOF TOP UNITS ARE APPROXIMATELY LOCATED. EXACT SIZE AND LOCATIONS SHALL BE COORDINATED WITH THE SUCCESSFUL MECHANICAL CONTRACTOR. ALL ROOF WALL OPENINGS SHALL BE SUPPORTED WITH TYPICAL ANGLE FRAME AND PENETRATION/SKROUD DETAILS.
 - a. EF = EXHAUST FAN
 - b. RH = ROOF HOOD
 11. DETAILING FOR CAST IN PLACE CONSTRUCTION ALLOWS FOR SHEAR WALLS AND COLUMNS TO BE PLACED VERTICALLY BEFORE PLACING ADDITIONAL FRAMING. THIS WILL REQUIRE TEMPORARY BRACING OF VERTICAL 10" WALLS UNTIL ROOF LEVELS ARE PLACED. COORDINATE BRACING OF WALLS WITH ARCHITECT IF BRACING TO EXPOSED STRUCTURE.
 12. MECHANICAL OPENINGS SHALL NOT OCCUR WITHIN 24" OF EMBEDDED STUD ANCHORS.

FRAMING PLAN LEGEND:



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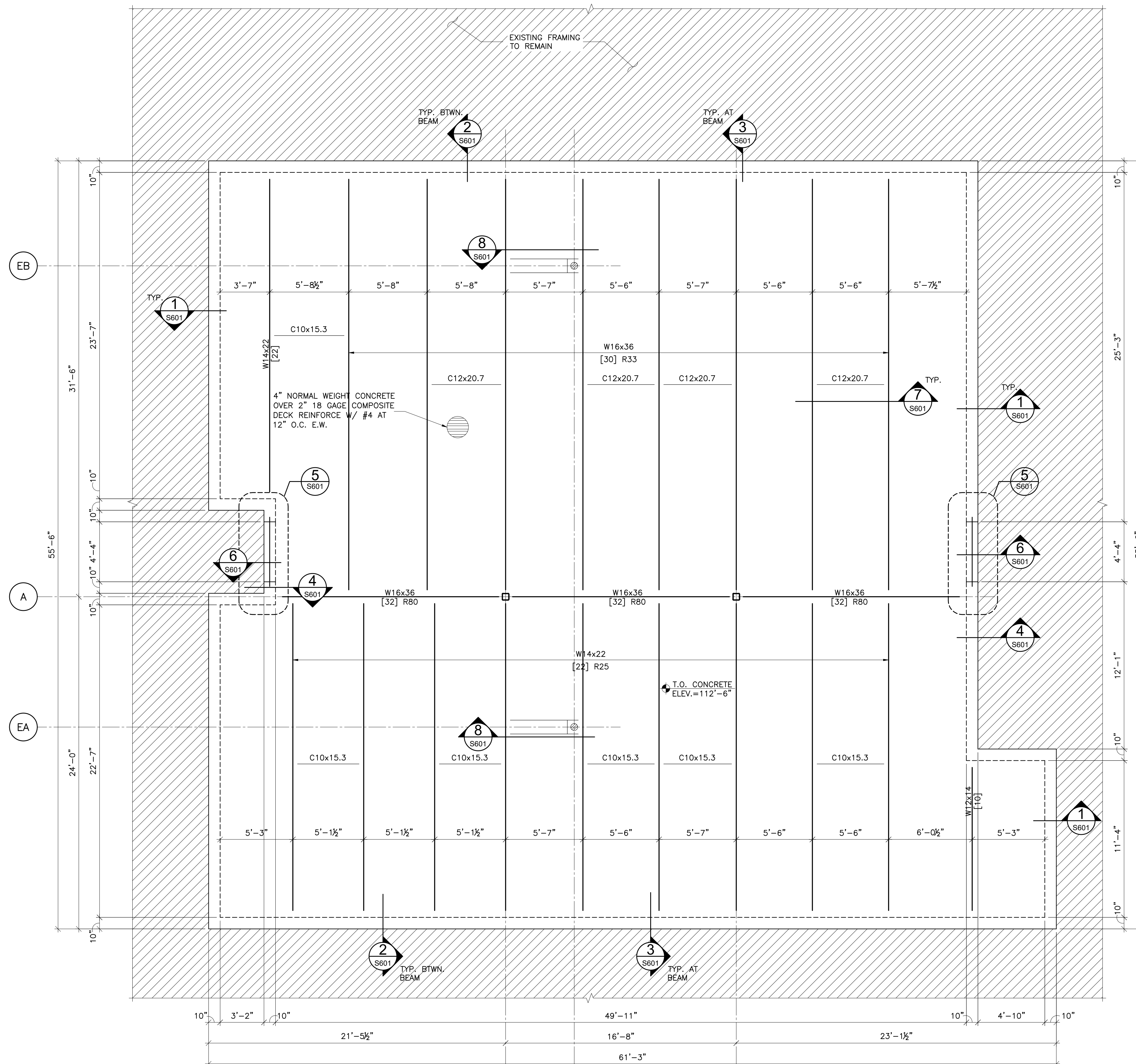
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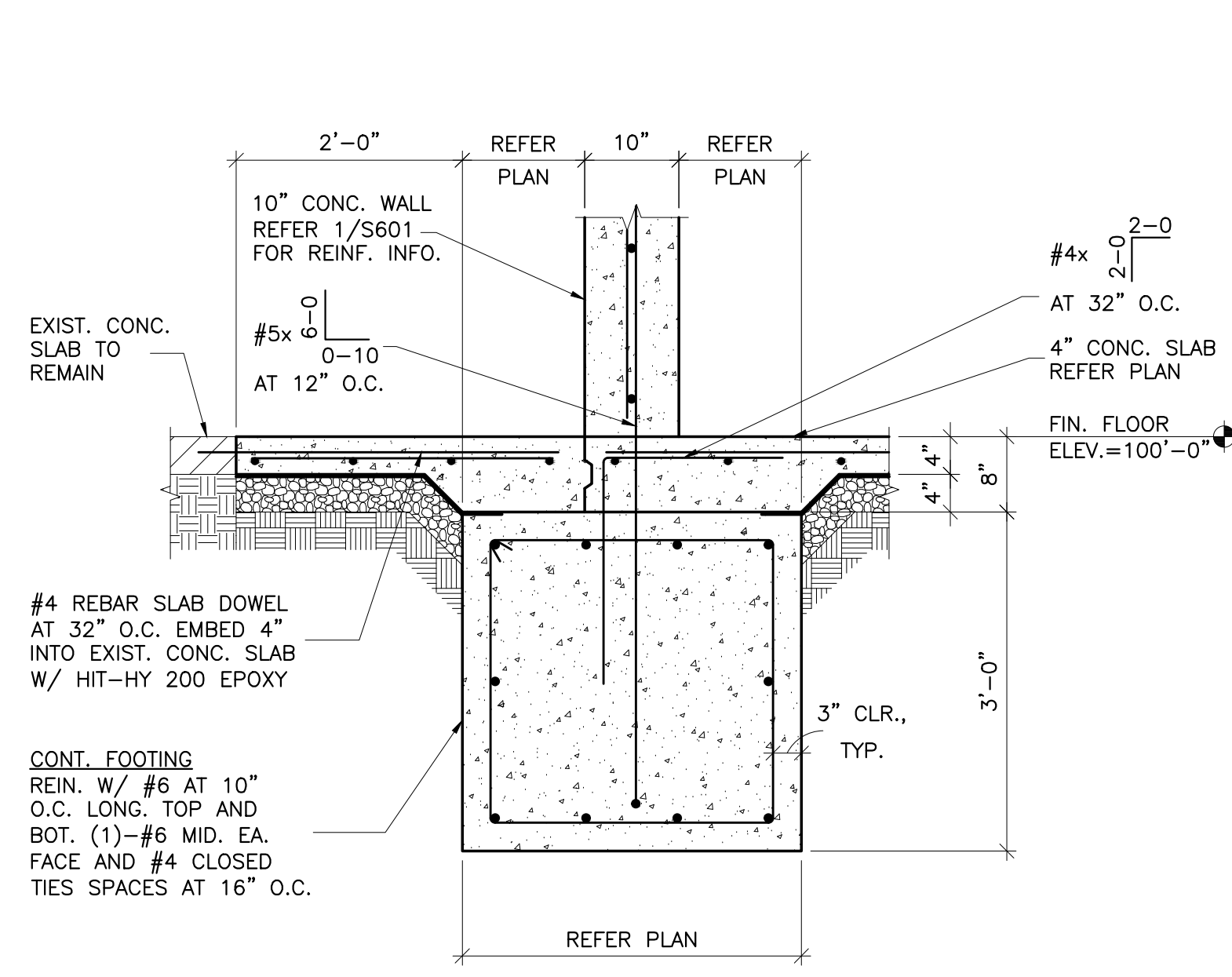


1 FRAMING PLAN
SCALE: 1/4"=1'-0"

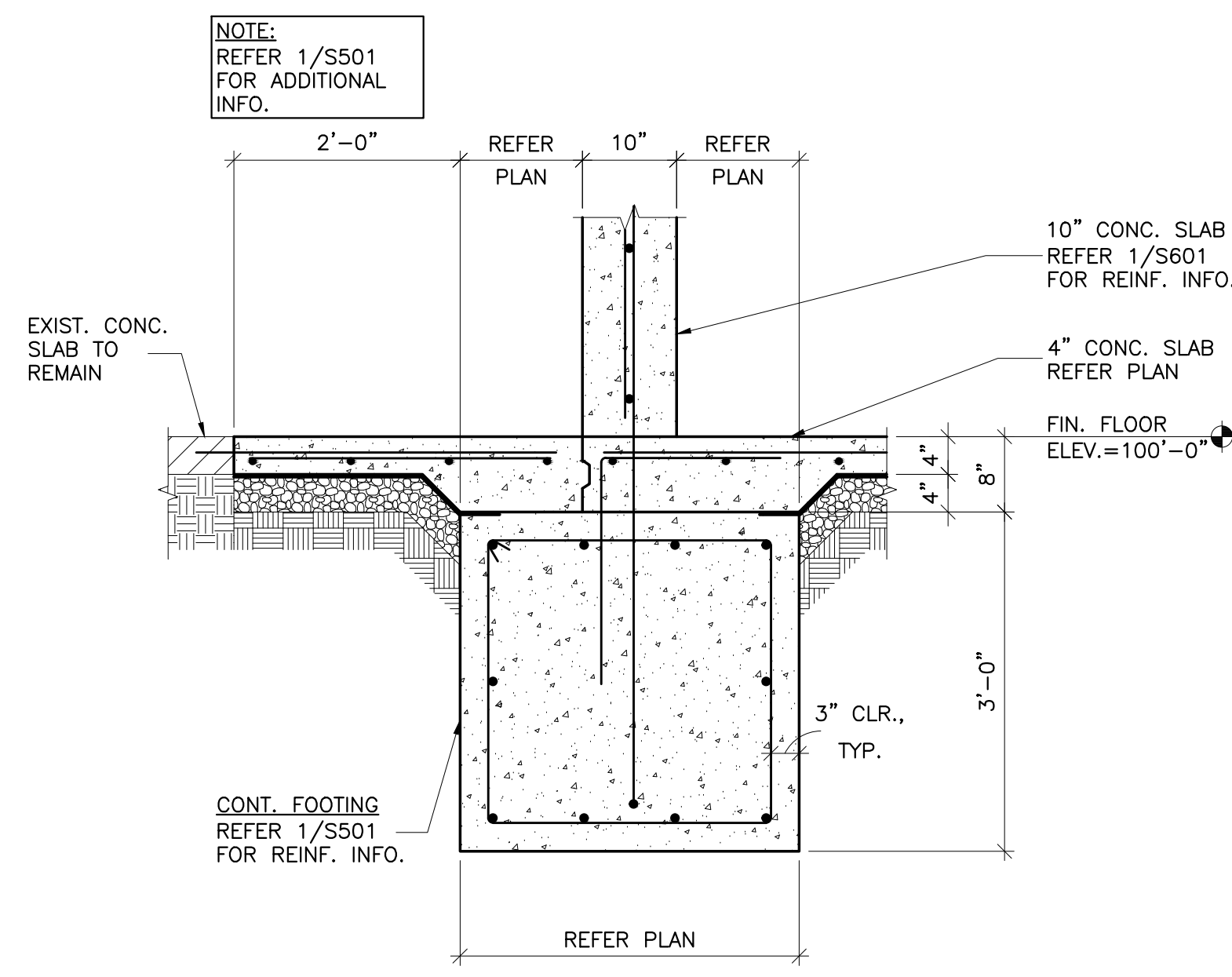
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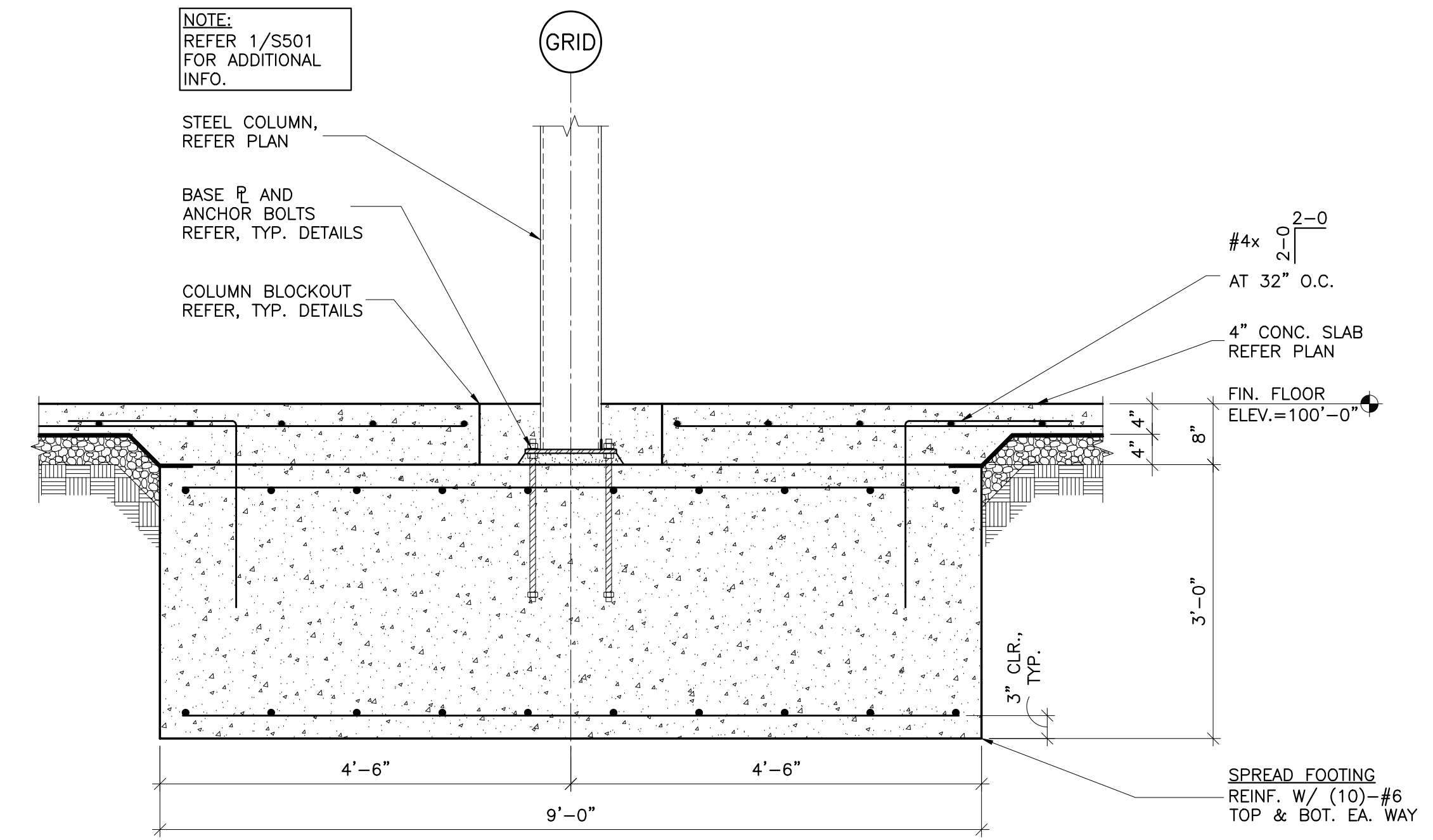
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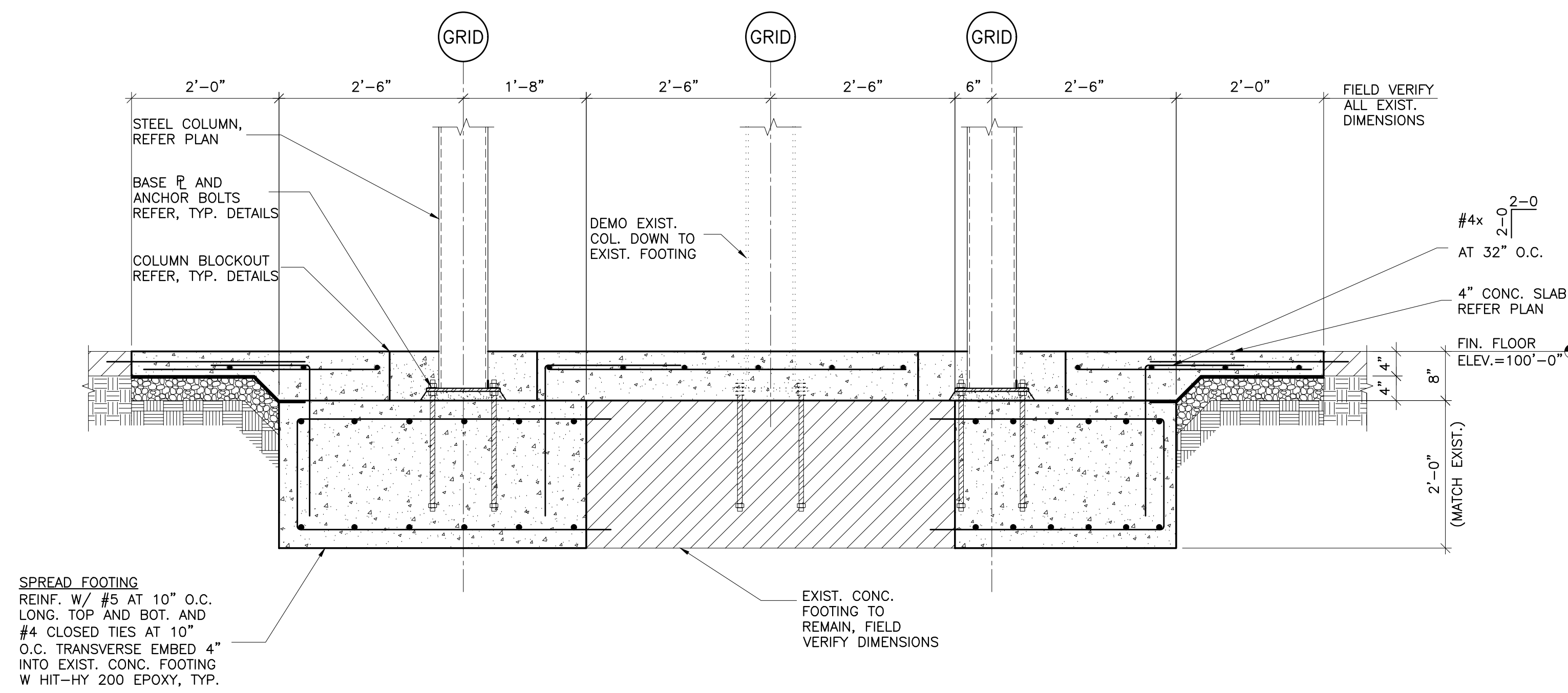
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S501 SCALE: 3/4"=1'-0"



2 SECTION
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3 SECTION
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4 SECTION
S501 SCALE: 3/4"=1'-0"

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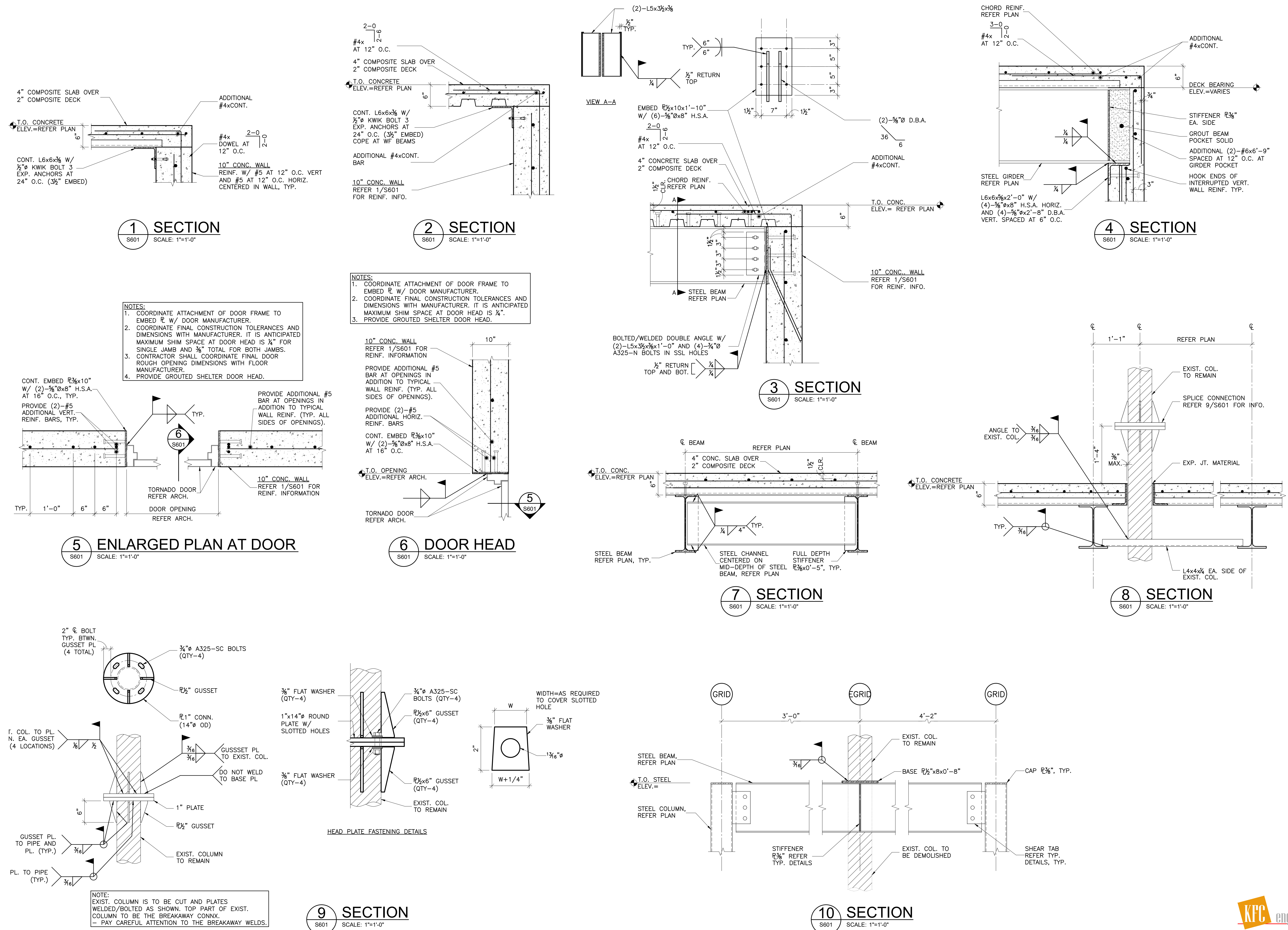
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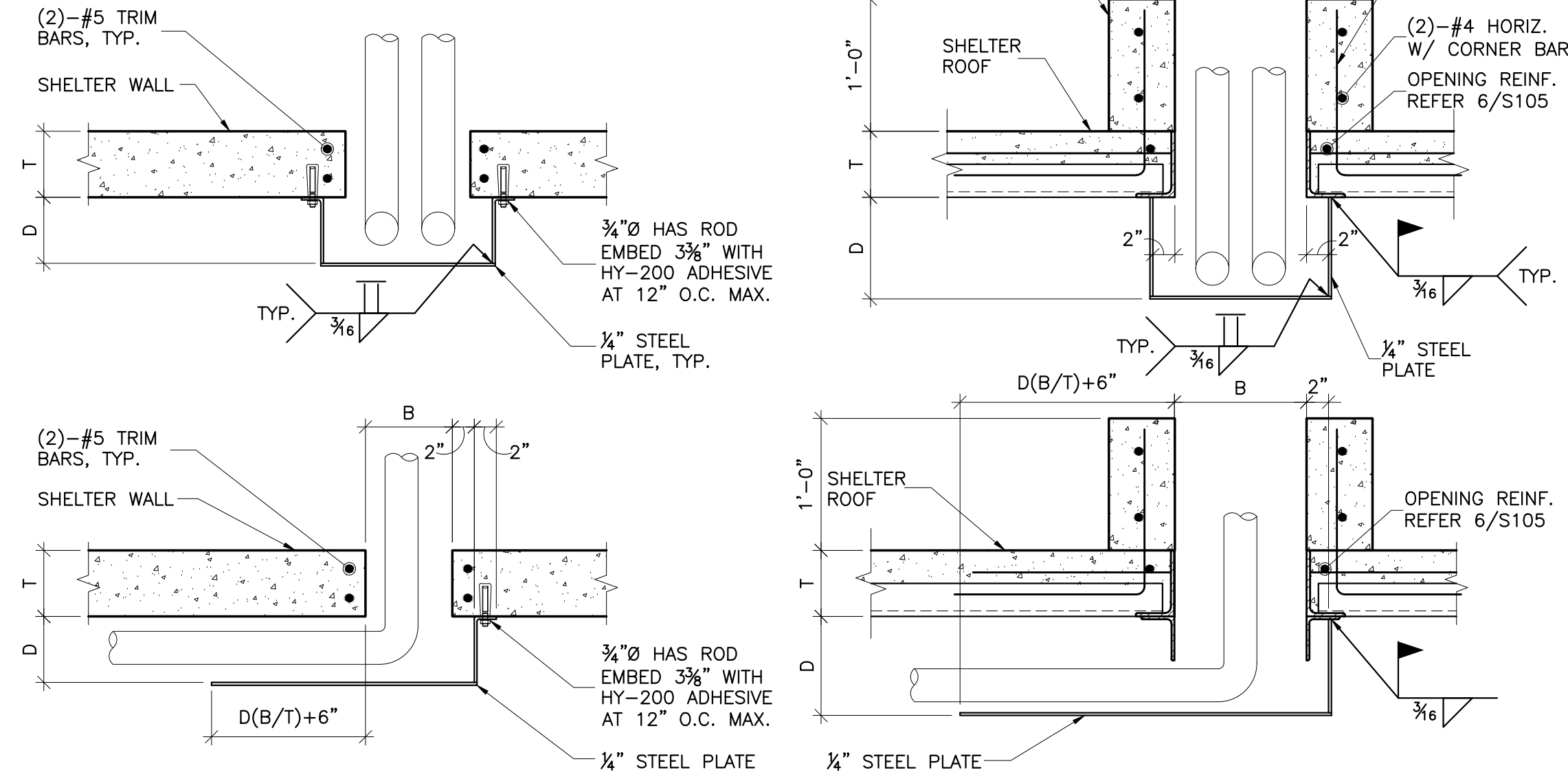
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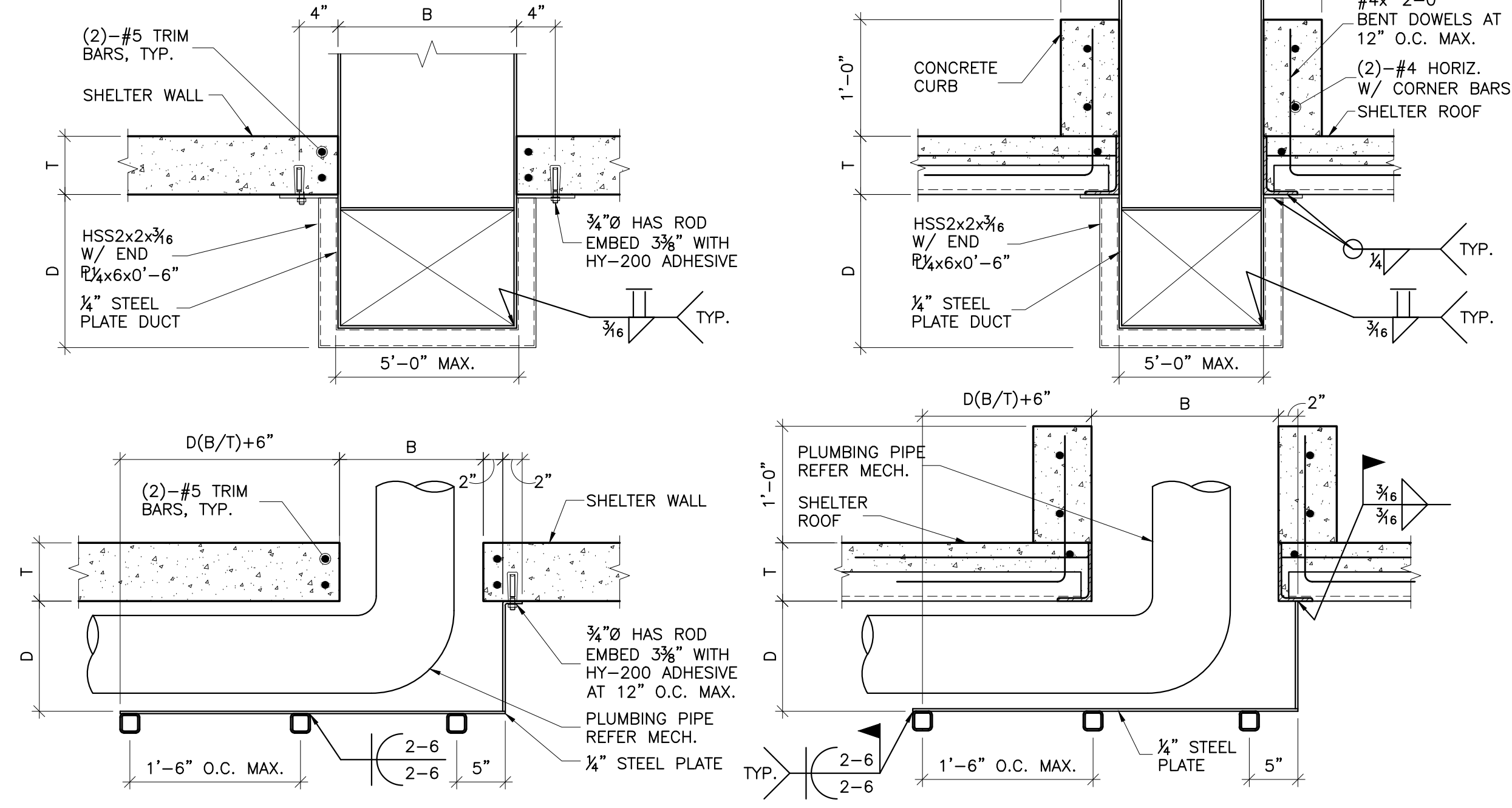
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NOTES:
1. WE ARE NOT AWARE OF ANY OPENINGS LARGER THAN 5'-0", IF AN OPENING LARGER THAN 5'-0" IS REQUIRED, CONTACT ENGINEER IMMEDIATELY FOR EVALUATION AND FURTHER INSTRUCTIONS.
2. REFER 6/S105 AND 7/S105 FOR ADDITIONAL INFORMATION.
3. DO NOT CUT WALL REINF. FOR INSTALLATION OF POST-INSTALLED ANCHORS.

NOTES:
1. OPENINGS 18" OR LESS MAY BE MADE IN THE SHELTER WALLS OR ROOF AS SHOWN.
2. REFER 6/S105 AND 7/S105 FOR ADDITIONAL INFORMATION.
3. DO NOT CUT WALL REINF. FOR INSTALLATION OF POST-INSTALLED ANCHORS.

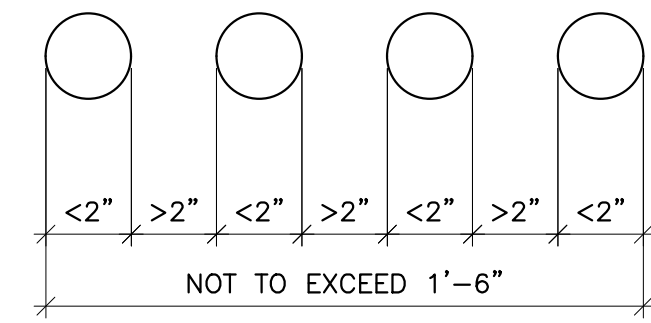


2
OPENINGS IN SHELTER 2" TO 1'-6"
SCALE: 1"=1'-0"



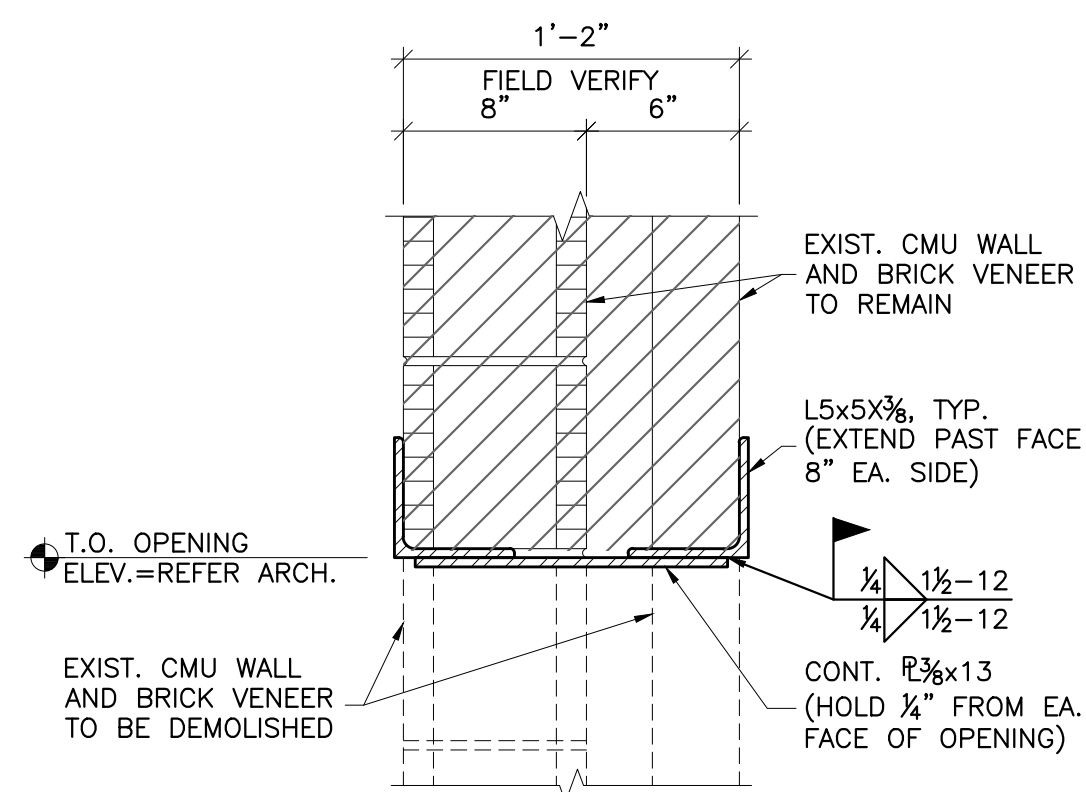
3
OPENINGS IN SHELTER TO 1'-6" TO 5'-0"
SCALE: 1"=1'-0"

NOTE:
OPENINGS 2" OR LESS MAY BE MADE IN THE SHELTER WALLS OR ROOF WITHOUT PROTECTION OR REGARD TO THE TYPICAL REINFORCING (SPECIAL REINFORCING AROUND OPENINGS SHALL NOT BE CUT). GROUPS OF UP TO 4 OPENINGS 2" OR LESS MAY BE MADE PROVIDED THE CLEAR SPACE BETWEEN OPENINGS EXCEEDS 2" AND THE TOTAL LENGTH OF THE GROUP DOES NOT EXCEED 18". OPENINGS CAN BE HORIZONTAL (AS SHOWN) OR VERTICAL.



1
OPENINGS IN SHELTER 2" OR LESS
SCALE: 1"=1'-0"

SEQUENCING NOTES:
1. SAW-CUT HORIZONTAL SLOT INTO ONE SIDE OF EXISTING WALL FOR PLACEMENT OF NEW ANGLE. SAW-CUT SHALL EXTEND A MINIMUM OF 8" BEYOND NEW OPENING.
2. INSTALL NEW ANGLE TIGHT INTO SLOT. ANGLES SHALL EXTEND A MINIMUM OF 8" BEYOND OPENING.
3. REPEAT STEPS 1 AND 2 ON OPPOSITE SIDE OF WALL.
4. DEMOLISH MASONRY AND BRICK VENEER TO EXTENTS SPECIFIED BY ARCH., FOR NEW OPENING.
5. INSTALL BOTTOM PL TO WITHIN 1/4" OF EACH JAMB OF NEW OPENING.
6. PAINT ANY EXPOSED PORTIONS OF LINTEL, REFER ARCH FOR COLOR.



4
SECTION
SCALE: 1 1/2"=1'-0"

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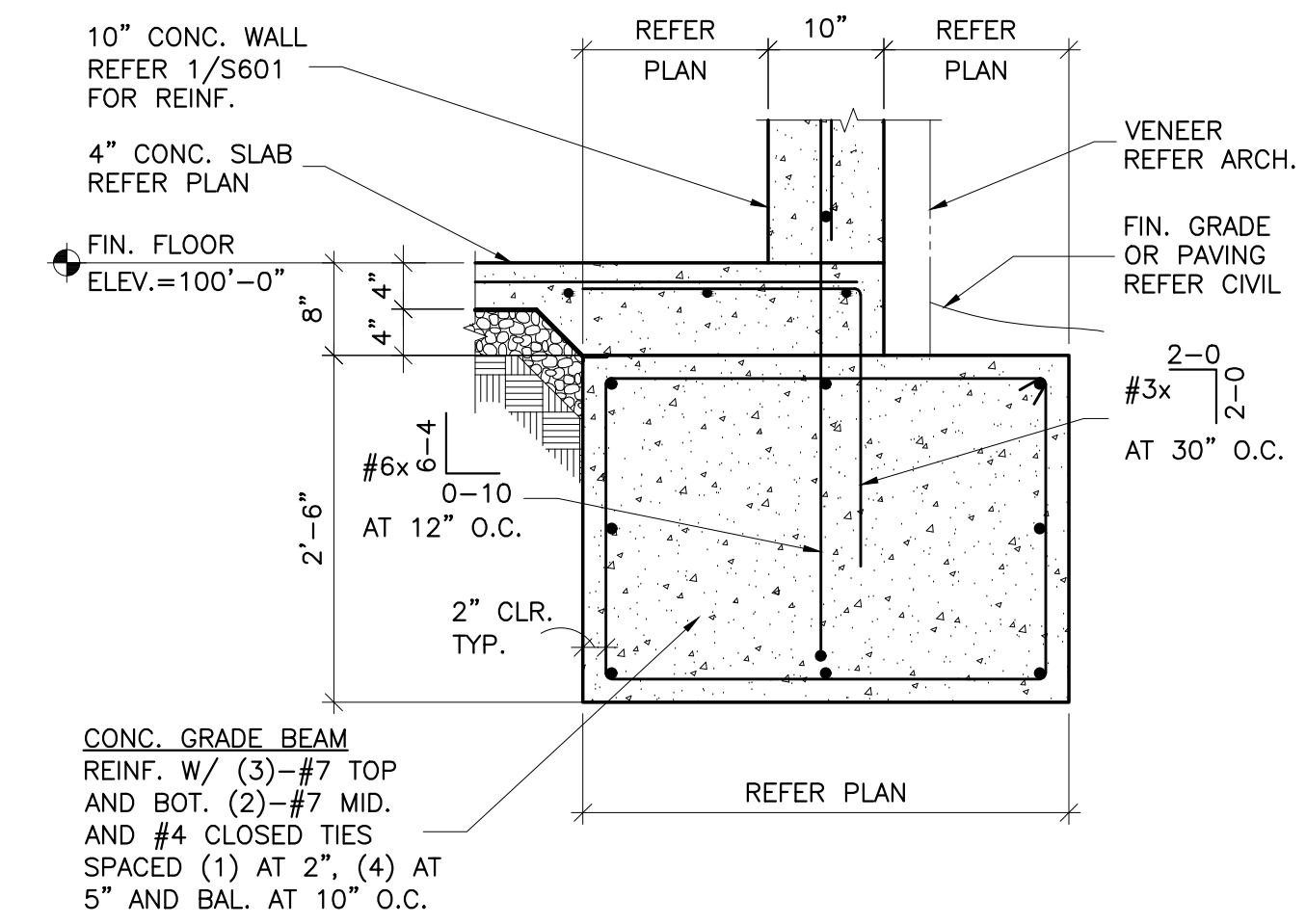
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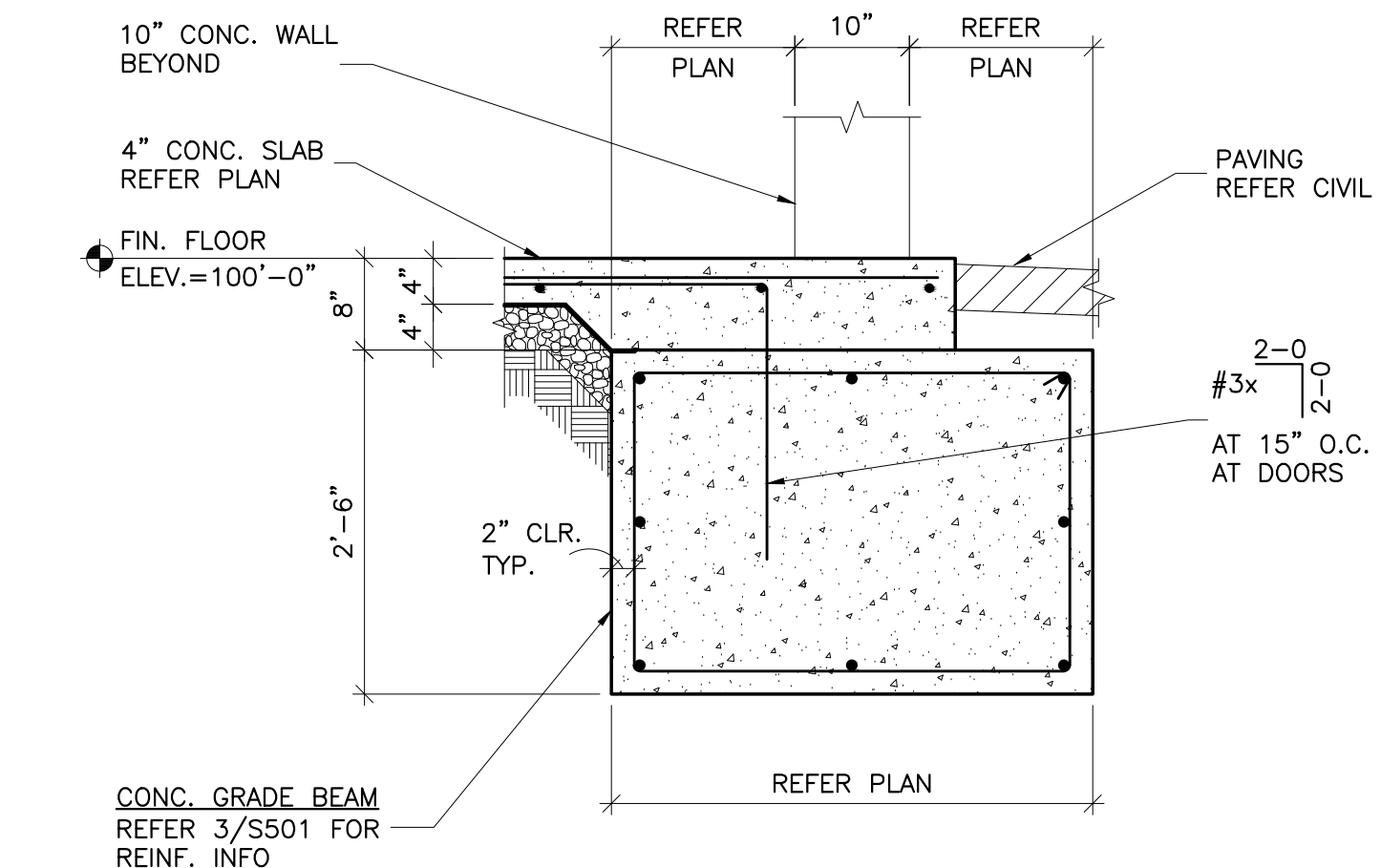
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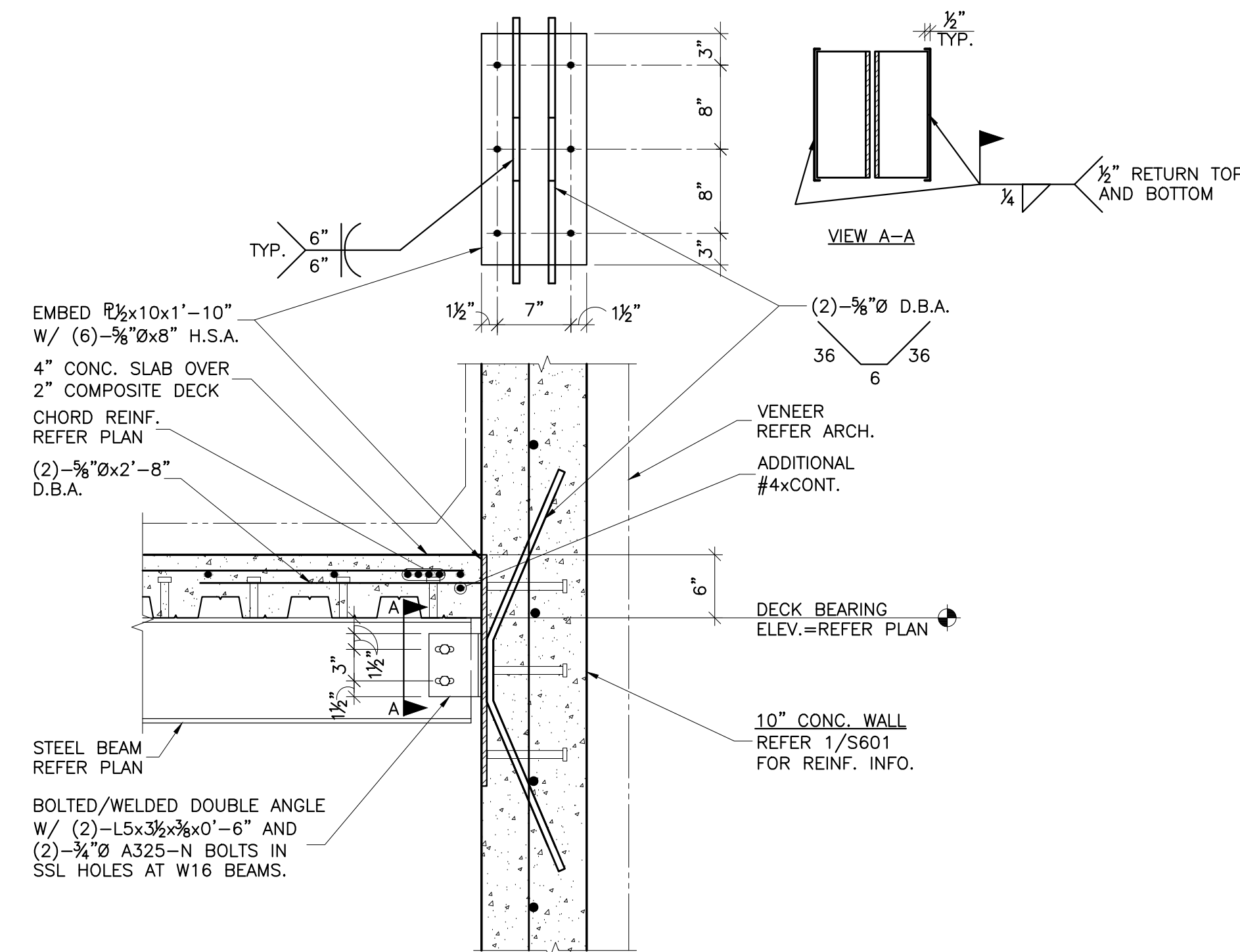
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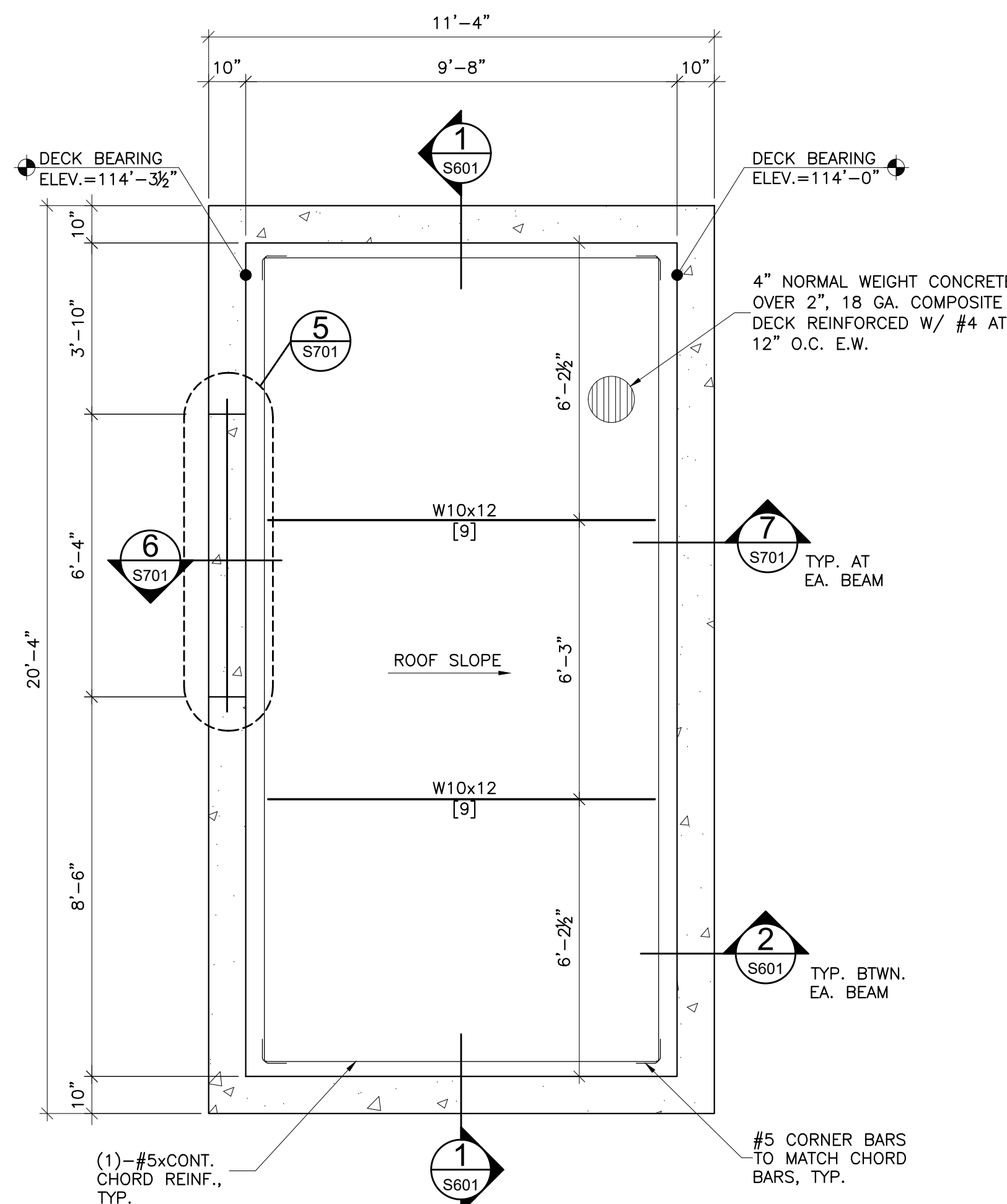
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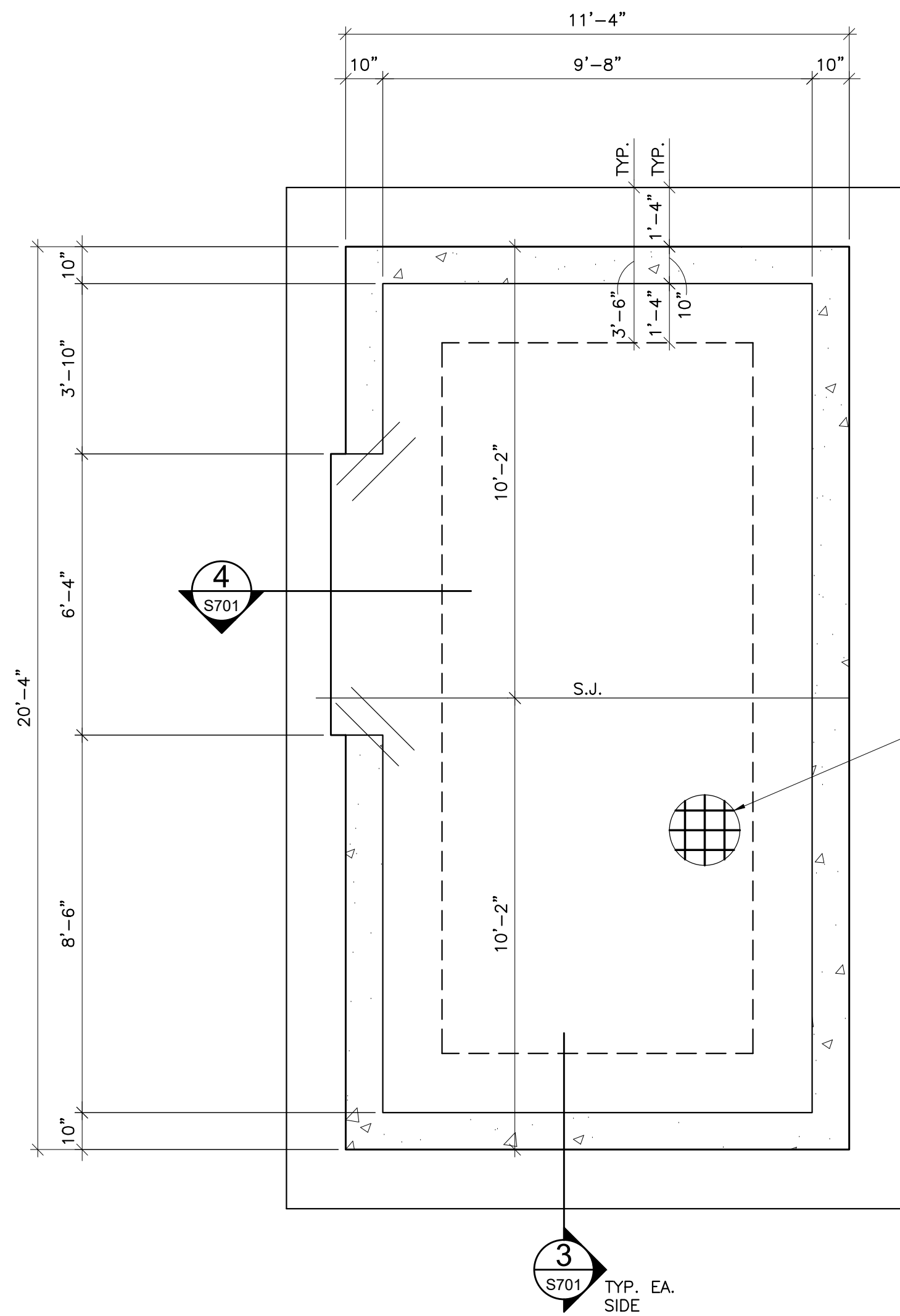
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7 SECTION
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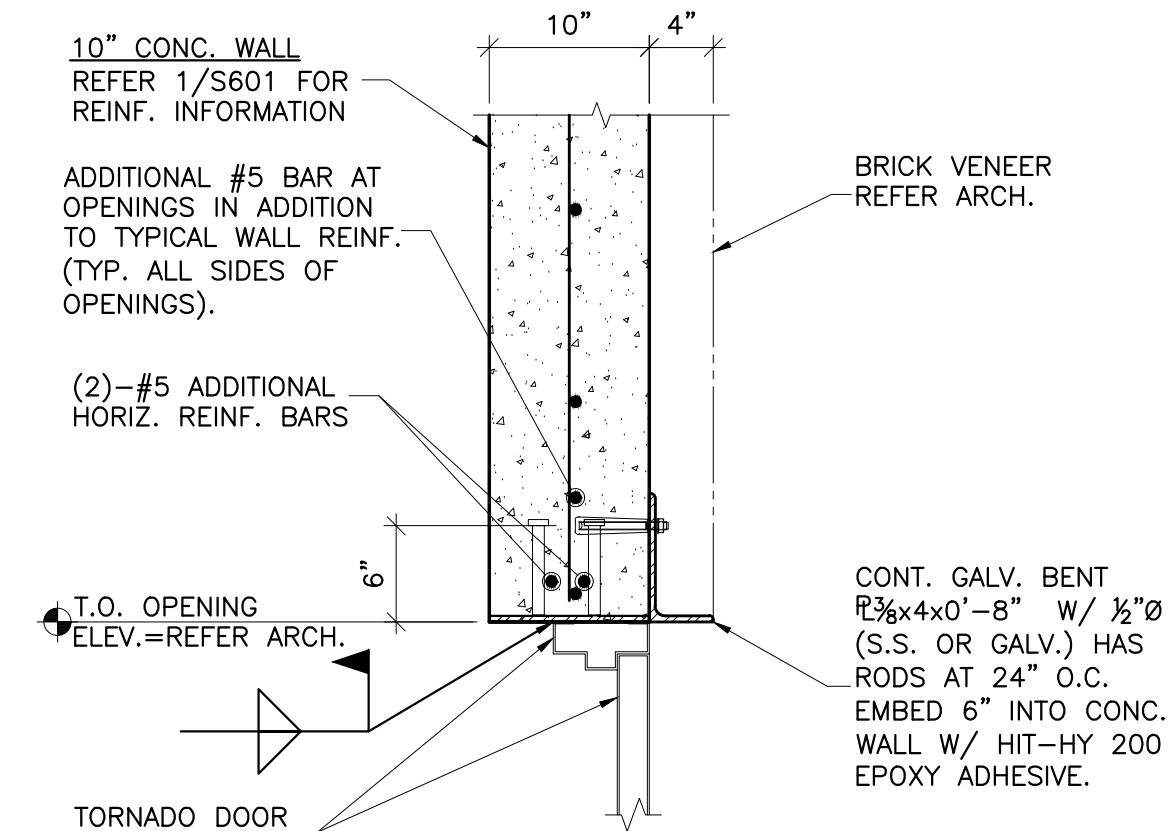


2 GENERATOR FRAMING PLAN
S701 SCALE: 3/8"=1'-0"



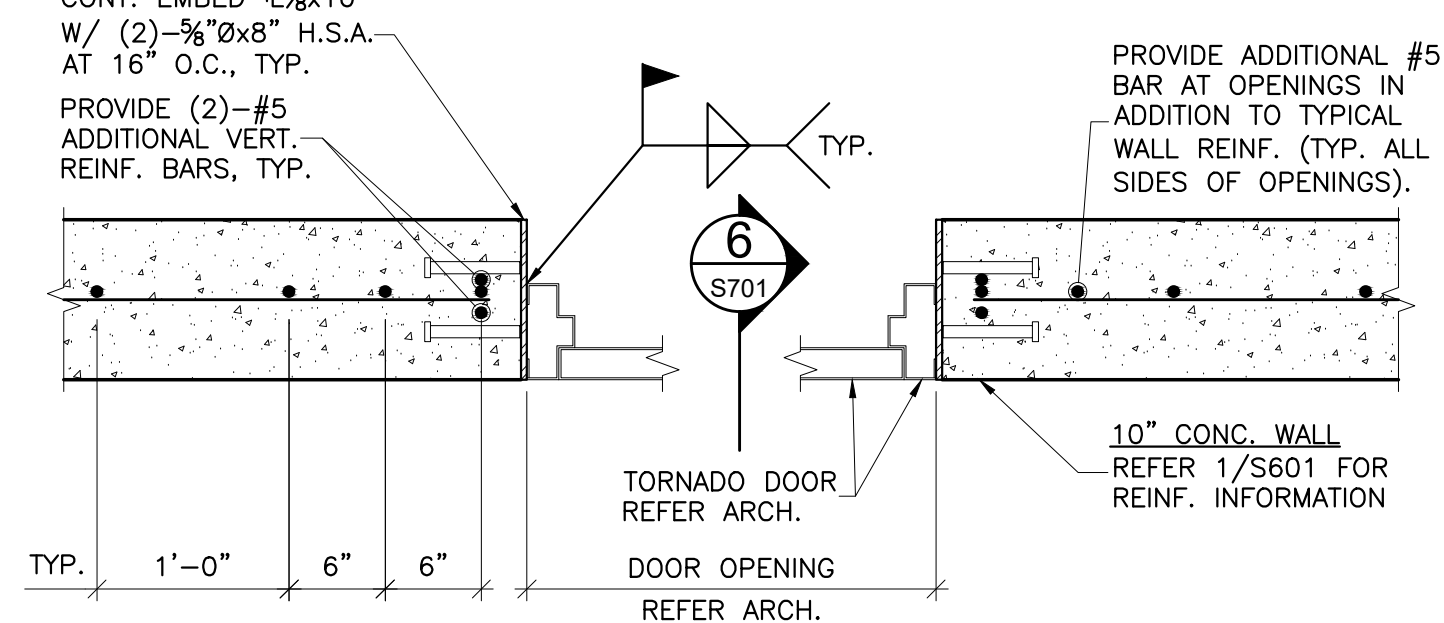
1 GENERATOR FOUNDATION PLAN
S701 SCALE: 3/8"=1'-0"

- NOTES:
- DO NOT CUT WALL REINF. FOR INSTALLATION OF POST-INSTALLED ANCHORS.
 - COORDINATE ATTACHMENT OF DOOR FRAME TO EMBED #5 W/ DOOR MANUFACTURER
 - COORDINATE FINAL CONSTRUCTION TOLERANCES AND DIMENSIONS WITH MANUFACTURER. IT IS ANTICIPATED MAXIMUM SHIM SPACE AT DOOR HEAD IS 1/4".
 - PROVIDE GROUTED SHELTER DOOR HEAD.



6 DOOR HEAD
S701 SCALE: 1"=1'-0"

- NOTES:
- COORDINATE ATTACHMENT OF DOOR FRAME TO EMBED #5 W/ DOOR MANUFACTURER.
 - COORDINATE FINAL CONSTRUCTION TOLERANCES AND DIMENSIONS WITH MANUFACTURER. IT IS ANTICIPATED MAXIMUM SHIM SPACE AT DOOR HEAD IS 1/4" FOR SINGLE JAMB AND 3/8" TOTAL FOR BOTH JAMBS.
 - CONTRACTOR SHALL COORDINATE FINAL DOOR ROUGH OPENING DIMENSIONS WITH FLOOR MANUFACTURER.
 - PROVIDE GROUTED SHELTER DOOR HEAD.



5 ENLARGED PLAN AT DOOR
S701 SCALE: 1"=1'-0"